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MONTANA RESOURCES

YANKEE DOODLE TAILINGS IMPOUNDMENT -LIFE OF MINE DESIGN REPORT FOR 6,560 AMENDMENT DESIGN DOCUMENT

Rev	Description	Date	
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EXECUTIVE SUMMARY

Montana Resources, LLC (MR) is in the process of preparing a permit amendment application (the 6,560 Amendment Application) for continued development of the Yankee Doodle Tailings Impoundment (YDTI) above the currently permitted maximum embankment crest elevation of 6,450 ft to facilitate continued operation of the mine after approximately 2034. The YDTI will continue to provide secure storage of mine tailings resulting from on-going mine operations. The YDTI embankments will continue to be progressively constructed with non-ore rockfill generated during open pit development.

The principal objectives for the YDTI design are to:

- Provide secure tailings and operating pond storage.
- Progressively improve the surface reclamation potential of the YDTI and surrounding facilities.
- Protect regional groundwater and surface waters from further impact.

The ongoing development and operation of the YDTI considers continuously achieving four key performance objectives as fundamental requirements for maintaining consistency with the design of the facility. These objectives incorporate the following:

- The YDTI supernatant pond remains separated from the embankment by large tailings beaches.
- The embankments and adjacent tailings beaches remain well drained, and piezometric elevations within the embankments and foundation remain below prescribed levels.
- Sufficient freeboard is maintained at all times to manage risks associated with extreme flood and seismic events.
- The embankment geometry, including downstream slope angle and crest width, remains consistent with design criteria.

The continued development of the YDTI will consider the following requirements:

- Phased development of the facility over the life of the mine.
- Construction material provided by mining the Continental Pit to the maximum practical extent, with the balance provided from external borrow areas, if required, to meet long-term objectives.
- Construction of the YDTI embankments using similar techniques, equipment, and construction methodologies that have been adopted for previous raises.
- The inclusion of monitoring features to confirm performance goals are achieved and design criteria are met.

The 6,560 Amendment Application considers the 2022 End of Year Reserve Report (Reserve Report), produced by MR, as the basis for evaluation of the long-term tailings and rockfill storage requirements. Knight Piésold Ltd (KP) prepared this report to outline the approximate sequencing for the continued development of the YDTI embankments and adjacent rock disposal sites (RDS). The sequencing of YDTI is presented in six phases considering the projected rockfill volumes, tailings filling schedule, and layout criteria, such as minimum crest widths and overall slopes. The six phases are as follows:

- 1. Phase 1: Ongoing construction activities to be completed prior to the permit amendment (Prior to Permit)
- 2. Phase 2: EL. 6,500 ft Embankment Crest Raise
- 3. Phase 3: EL. 6,500 ft Lower Embankment Lifts



- 4. Phase 4: EL. 6,500 ft Embankment Lifts and RDS Expansions
- 5. Phase 5: EL. 6,560 ft Embankment Crest Raise
- 6. Phase 6: Final Life of Mine (Prior to Closure)

Issue for Construction (IFC) designs for the various development stages will be progressively issued during ongoing mine development. The approximate YDTI development sequencing presented in this report will be updated as required for the IFC designs, and activities attributed to the six phases presented above may be adjusted or combined in the future as part of the ongoing development process. Expansion of the instrumentation and monitoring network around the YDTI and within the RDS areas will be progressively required throughout continued development of the facility.

Preliminary Quantitative Performance Parameters (QPPs) were developed for ongoing operations and facility closure. Active QPPs are formally presented in the Tailings Operations, Maintenance and Surveillance (TOMS) Manual. The QPPs are regularly reviewed and revised when required as part of annual updates to the TOMS Manual. The preliminary QPPs presented in this report may be incorporated into future versions of the TOMS Manual, when appropriate, and updated thereafter during regular reviews of the TOMS Manual.



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APPENDICES

Appendix A Tailings Deposition Modelling and Storage Capacity Evaluation
Appendix B Design Drawing Package - 6,500 ft Embankment Crest
Appendix C Design Drawing Package - 6,560 ft Embankment Crest



ABBREVIATIONS

ACC	Anaconda Copper Company
BMFOU	Butte Mine Flooding Operable Unit
CMP	
EL	elevation
EOR	Engineer of Record
EPA	Environmental Protection Agency
ft	feet
GPS	Global Positioning System
GNSS	Global Navigation System Satellite
H:V	horizontal to vertical
HsB	Horseshoe Bend
IFC	Issued For Construction
InSAR	interferometric synthetic aperture radar
IRP	Independent Review Panel
KP	Knight Piésold Ltd.
MCA	Montana Code Annotated
MDEQ	Montana Department of Environmental Quality
MR	Montana Resources, LLC
Mtons	million tons
N	North
NW	Northwest
QPPs	
RDS	rock disposal sites
RMS	remote monitoring system
SOL	setting out line
SWD	surface water ditch
TAC	The Anaconda Company
the Design Document	6,560 Amendment Design Document
TOMS	Tailings Operations, Maintenance and Surveillance
VWP	vibrating wire piezometer
W	West
WED	West Embankment Drain
YDTI	Yankee Doodle Tailings Impoundment



1.0 INTRODUCTION

1.1 LOCATION

Montana Resources, LLC (MR) operates the Montana Resources open pit copper and molybdenum mine located in Butte, Montana. The ore throughput at the mill and processing facilities is approximately 49,000 short tons per day. The tailings from ore processing are conveyed to the Yankee Doodle Tailings Impoundment for disposal and permanent storage. The mine is located in Butte, Silver Bow County and is bounded by Interstate 15 and the Continental Divide on the east, Moulton Reservoir Road on the west, and Farrell Street, Continental Drive and Shields Avenue to the south.

The key components of the MR facilities, as shown on Figure 1.1, include the following:

- Yankee Doodle Tailings Impoundment (YDTI)
- Berkeley Pit
- Continental Pit
- Mill and processing facilities (the Concentrator)
- Horseshoe Bend (HsB) Area and associated facilities

1.2 PURPOSE AND SCOPE

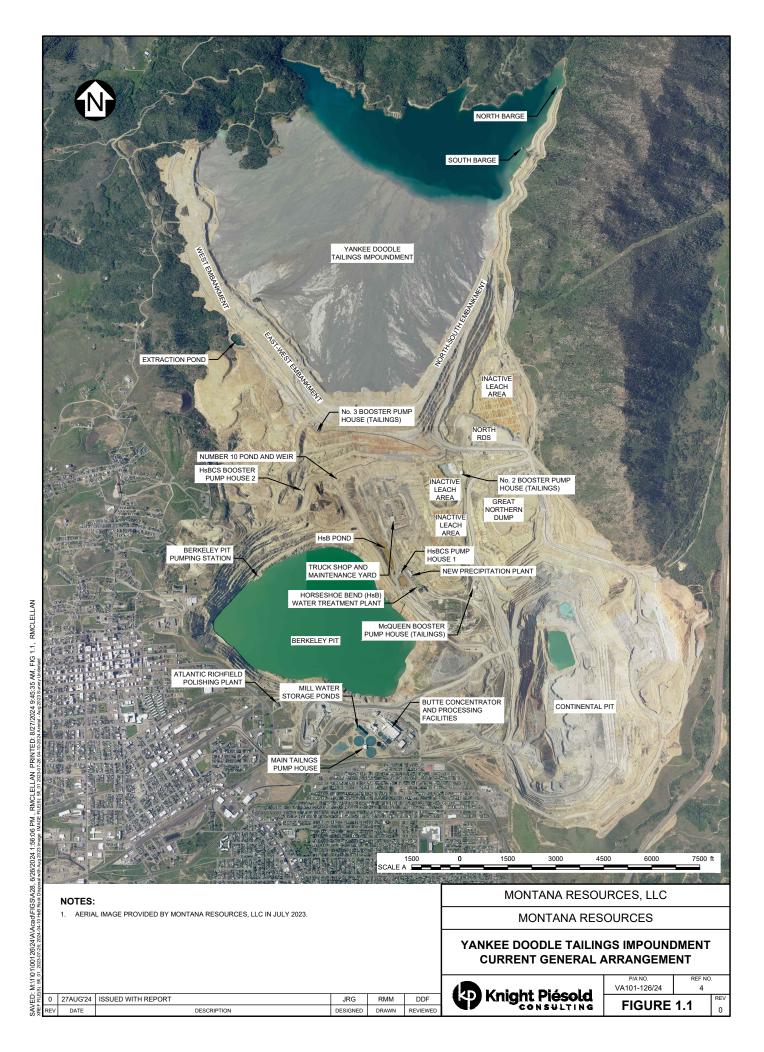
The YDTI is currently permitted to a maximum crest elevation (EL.) of 6,450 feet (ft). The EL. 6,450 ft embankment provides sufficient tailings storage capacity to support mining and ore processing until approximately 2034. MR is preparing a permit amendment application (the 6,560 Amendment Application) to facilitate continued operation of the mine thereafter by aligning approval for tailings storage at the YDTI with the remaining ore reserves. The permit amendment application process requires the permit applicant (MR) to submit a design document when expansion of an existing facility is proposed.

Knight Piésold Ltd. (KP) is developing the 6,560 Amendment Design Document (the Design Document) to support the 6,560 Amendment Application. The Design Document presents the plan to progressively raise the crest elevation of the YDTI embankments to a maximum design crest of EL. 6,560 ft in two or more lifts to support continued mining and ore processing. The Design Document comprises a series of technical reports covering the subject areas and content to meet the requirements specified in Montana State law as well as evaluating opportunities for continued risk reduction to enhance safety as part of the fundamental objective for on-going continuous improvement of the safety of the YDTI. The laws governing tailings storage facility design, operation and reclamation are contained within sections of Montana Code Annotated (MCA) Title 82 Chapter 4 Part 3 (MCA, 2023).

- Title 82: Minerals, Oil, and Gas
 - Chapter 4: Reclamation
 - Part 3: Metal Mine Reclamation

This report has been prepared by KP to outline the proposed life of mine development of the YDTI embankments and rock disposal sites (RDS) up to EL. 6,560 ft. The geotechnical analyses for select stages of YDTI development are presented in a standalone stability assessment report (KP, 2024a).





1.3 COORDINATE SYSTEM

The YDTI references the site coordinate system known as the 'Anaconda Mine Grid' established by The Anaconda Company (TAC) in 1957. The Anaconda Mine Grid is based on the Anaconda Copper Company (ACC) Datum established in 1915. All elevations are stated in Anaconda Mine Grid coordinates with respect to the ACC Vertical Datum unless specifically indicated otherwise. The Montana Resources Global Positioning System (GPS) Site Coordinate System is based on the 'Anaconda Mine Grid' and utilizes International Feet.



2.0 DEVELOPMENT OBJECTIVES AND HISTORICAL PRECEDENTS

2.1 OBJECTIVES

The YDTI will continue to provide secure storage of mine tailings resulting from on-going mine operations, with construction materials provided from pit stripping. The principal design objectives for the YDTI are to:

- Provide secure tailings and operating pond storage.
- Progressively improve the surface reclamation potential of the YDTI and surrounding facilities.
- Protect regional groundwater and surface waters from further impact.

The ongoing development and operation of the YDTI considers continuously achieving four key performance objectives as fundamental requirements for maintaining consistency with the design of the facility. These objectives incorporate the following:

- The YDTI supernatant pond remains separated from the embankment by large tailings beaches.
- The embankments and adjacent tailings beaches remain well drained, and piezometric elevations within the embankments and foundation remain below prescribed levels.
- Sufficient freeboard is maintained at all times to manage risks associated with extreme flood and seismic events.
- The embankment geometry, including downstream slope angle and crest width, remains consistent with design criteria.

Active Quantitative Performance Parameters (QPPs), including the prescribed piezometric elevations referenced above, are formally presented in the Tailings Operations, Maintenance and Surveillance (TOMS) Manual. The QPPs are regularly reviewed and revised when required as part of annual updates to the TOMS Manual.

The development of the YDTI embankments and adjacent RDS considers the design and operational objectives outlined above with the goal of continuous improvement in safety and enhancement of stability through slope flattening and progressive buttressing of the facility embankments. These progressive improvements are made possible by continued mining at the site. The development of the YDTI also considers the following requirements:

- Phased development of the facility over the life of the mine.
- Construction material provided by mining the Continental Pit to the maximum practical extent, with the balance provided from external borrow areas, if required, to meet long-term objectives.
- Construction of the YDTI embankments using similar techniques, equipment, and construction methodologies that have been adopted for previous raises.
- The inclusion of monitoring features to confirm performance goals are achieved and design criteria are met.

2.2 LEGACY CROSS SECTION CONVENTION

The design of the YDTI references a series of legacy cross-section locations that have been historically used for the project. These cross-sections most likely align with a historical setting out line for the



embankments that is no longer consistent with the current arrangement; however, the legacy cross-sections are used for the on-going layout in the interest of consistency for as-built drawings and annual reporting. The convention begins with Station 0+00 at the interface between the North-South and East-West Embankments and increases in station number in both directions. The stationing convention uses a directional suffix (e.g., N, NW, or W) to describe the location of the cross-section (e.g., 8+00 N for 800 ft along the North-South Embankment). The actual stationing measured along the current and proposed setting out line (SOL) may not be equal to the stationing as referenced on the cross-section due to differences in the historical and current/proposed setting out lines.

2.3 EXISTING YDTI EMBANKMENTS

The YDTI was originally constructed in 1963 using rockfill from the Berkeley Pit and has been continuously constructed to EL. 6,450 ft using rockfill from the Berkeley Pit (until 1982) and from the Continental Pit (beginning in 1986). The YDTI comprises a valley-fill style impoundment created by a continuous rockfill embankment that for descriptive purposes is divided into three rockfill embankments according to the general geometry of each limb of the continuous embankment. These embankments are the:

- North-South Embankment: The North-South Embankment forms the eastern to southeastern limb of
 the YDTI and runs approximately north to south in orientation. The North-South Embankment abuts
 onto the base of Rampart Mountain, forming the eastern battery limit of the MR mine site.
- East-West Embankment: The East-West Embankment forms the southwestern limb of the YDTI and runs approximately east to west in orientation. The East-West Embankment is constructed upstream of the HsB area and the Berkeley Pit.
- West Embankment: The West Embankment forms the western limb of the YDTI and runs approximately north to south in orientation. The West Embankment is constructed along the side of the West Ridge and forms the western battery limit of the facility. The West Embankment incorporates the West Embankment Drain (WED) and several other seepage control features, which will maintain hydrodynamic containment of YDTI seepage as the supernatant pond elevation rises above the lowest groundwater elevations in the West Ridge. The West Embankment also includes zone of material with relatively lower permeability and relatively lower acid generating potential (Zone D1) along the downstream edge of the West Embankment.

2.4 HISTORICAL CONSTRUCTION PRACTICES AND PRECEDENT

The YDTI has historically been constructed by progressively placing rockfill to form free-draining rockfill embankments. The rockfill comprises pit-run material historically end-dumped in approximately 30 to 100 ft lifts with initial compaction from the mine haul fleet traffic and gradual settlement occurring thereafter. Ripping of the embankment surface to enhance vertical infiltration has been commonly completed prior to subsequent fill placement.

The Amendment 10 permit application included design and construction specifications outlining subgrade preparation requirements and maximum 50-ft lift thicknesses for rockfill materials. The EL. 6,450 ft embankment design also continued to include a relatively finer-grained material (alluvium) placed on the upstream face of the embankment to limit potential for tailings migration into the rockfill.



The design presented herein for the Design Document is consistent with these historical practices and precedents. Refinements to alluvium placement methodology along the upstream face of the embankment are incorporated as part of the fundamental objective for on-going continuous improvement.

2.5 CONSTRUCTION MATERIALS

The embankment and RDS will generally be constructed using the similar materials and construction practices historically used on site and described below. Placement, compaction, and testing requirements for each material are outlined in the Construction Management Plan (CMP) (KP, 2024b). Specific material boundaries for each embankment are outlined in subsequent sections. Additional supporting materials may be specified during Issued For Construction (IFC) design and are not presented in this report (i.e. additional drainage materials).

- Zone U Rockfill: 'U' material will be sourced from the Continental Pit and used in various construction areas, including the YDTI embankments, RDS, and other general construction areas such as haul ramps and access roads. Zone U is intended to be constructed in a manner that promotes free draining behavior. 'U' material will typically be hauled and end-dumped in horizontal lifts up to approximately 50 ft thick using the 240-ton mine fleet haul trucks. Segregation will occur as the rockfill is end-dumped from the crest of each lift. The finer particles will tend to accumulate near the top of the lift and the cobbles and boulders roll further down the slope and accumulate at the toe. Zone U (Embankment) is delineated as the structural portion of the embankment, while Zone U (Surcharge) is delineated in areas where Zone U is located above historical tailings materials.
- Zone D1 Rockfill: 'D1' material will be sourced from the Continental Pit. Zone D1 is intended to have relatively lower acid generating potential (than Zone U) and lower permeability compared to 'U' material. 'D1' rockfill will be hauled and end-dumped by the 240-ton haul trucks and spread in approximately 5 ft thick horizontal lifts prior to compaction. 'D1' rockfill will typically be used to construct the downstream zone of the West Embankment to act as an impediment to horizontal migration of perched seepage flow towards the downstream face of the embankment. The primary design intent of Zone D1 is to encourage collection of seepage flows by the WED at the West Embankment.
- Zone F Earthfill: 'F' material will be dumped and spread by dozer to construct a separation zone between the tailings and Zone U on the upstream face of the embankment. The primary design intent of Zone F is to limit potential for tailings migration into zones of relatively coarse rockfill.
- Zone D2 Earthfill: 'D2' material will be placed to provide a capping layer on the downstream slope of
 the embankment to promote runoff of meteoric water and limit surface exposure of materials with higher
 acid potential (e.g. Zones U and D1). 'D2' material will typically comprise non-acid generating alluvium
 dumped by contractor truck fleets and spread by dozer.



3.0 BASIS OF FACILITY DEVELOPMENT

3.1 MATERIAL ESTIMATES

The Design Document considers the 2022 End of Year Reserve Report (2022 Reserve Report) produced by MR (MR, 2023) as the basis for evaluating the long-term tailings and rockfill storage requirements. The estimated tailings and rockfill tonnages from the 2022 Reserve Report are summarized in Table 3.1. The reserve report includes the total proven and probable reserves and is updated by MR each year based on project economics, resource drilling, and other mine operation decisions.

Table 3.1 Summary of the 2022 End of Year Reserve Report

	Quantity	
Material	Million Tons (MTons)	
Ore (Tailings)	570	
Rockfill	562	

Note(s):

1. The 2022 Reserve Report (MR, 2023) includes Continental Pit Area CE/CS/DN/DE/DS/EN pushbacks and the Central Zone.

3.2 TAILINGS STORAGE CAPACITY

Tailings deposition modelling to evaluate the storage capacity of the YDTI was updated to support the Design Document as summarized in Appendix A. The modelling indicates the following key timeframes, which were considered for the timing of the facility development:

- The currently permitted and constructed YDTI crest elevation of EL. 6,450 ft will provide sufficient tailings storage capacity until approximately December 2034.
- The embankment crest elevation will need to be raised prior to 2034 to continue to provide tailings storage for ongoing mine operations without interruption.
- The YDTI embankments constructed to the proposed EL. 6,560 ft will provide sufficient tailings storage capacity until the currently identified ore reserves are exhausted (approximately June 2056).

3.3 ROCKFILL SCHEDULE

The total rockfill volume and placement schedule for the RDS in the long-term is dependent on several factors, including MR's rockfill forecast schedule, pit stripping requirements, embankment rockfill material needs, cut-off grades due to fluctuating metal prices, other mine site uses, and the duration of continued mining operations at the site.

A mid-range rockfill mining schedule was produced by MR for the Continental Pit providing the estimated rockfill tonnages available for embankment and RDS construction from 2023 through 2041 (~189 Mtons). This schedule and the projected tailings filling schedule forms the basis of the estimated embankment and RDS construction schedule over that period of time. Short-range mine plans are updated at least annually by MR to set budget numbers and priorities for continued mining.



Longer-term rockfill scheduling, including mining of the Central Zone, has not been completed in detail. A summary of the rockfill quantities and their location within the pit development model, as presented in the 2022 Reserve Report, is provided in Table 3.2.

Table 3.2 2022 End of Year Reserve Report Rockfill Breakdown

Rockfill Location	Rockfill Quantity (Mtons)	Notes
CE/CS/DN Pushback	35	189 Mtons in mid-range mining
DE/DS/EN Pushback	284	schedule assumed to be sourced from these locations.
Central Zone Plan	243	
Total	562	

A summary of the estimated annual availability of rockfill used in the YDTI life of mine development plan presented in this report is shown in Table 3.3. The life of mine distribution considers the rockfill quantities presented within the mid-range mining schedule and the 2022 Reserve Report with assumptions related to the future timing of the rockfill mining for materials not contained in the mid-range mining schedule. It was assumed that pre-stripping for continued mining after 2041 will be completed over a 10-year period and begins five years prior to end of the mid-range schedule (i.e., in 2037). The balance of the CE/CS/DN and DE/DS/EN pushbacks remaining after the mid-range schedule was assumed for the pushback striping volume (~130 Mtons) and was evenly distributed over a 10-year period. The Central Zone rockfill mining was assumed to be evenly distributed from 2042 through the life of mine (i.e., through 2056 as per the tailings filling schedule).



Table 3.3 Estimated Rockfill Schedule

		Rockfill Quantity (Mtons)					
Year	Mid-Range Mine Schedule	Pushback Stripping	Central Zone Mining	Total Annual Rockfill	Total Cumulative Rockfill		
2023 ¹	19	-	-	19	19		
2024 ¹	25	-	-	25	44		
2025	22	-	-	22	66		
2026	19	-	-	19	85		
2027	17	-	-	17	102		
2028	16	-	-	16	118		
2029	18	-	-	18	136		
2030	18	-	-	18	154		
2031	6	-	-	6	160		
2032	4	-	-	4	164		
2033	2	-	-	2	166		
2034	2	-	-	2	168		
2035	1	-	-	1	169		
2036	2	-	-	2	171		
2037	4	13	-	17	188		
2038	3	13	-	16	204		
2039	4	13	-	17	221		
2040	4	13	-	17	238		
2041	3	13	-	16	254		
2042	-	13	16	29	283		
2043	-	13	16	29	312		
2044	-	13	16	29	341		
2045	-	13	16	29	370		
2046	-	13	16	29	399		
2047	-	-	19	19	418		
2048	-	-	16	16	434		
2049	-	-	16	16	450		
2050	-	-	16	16	466		
2051	-	-	16	16	482		
2052	-	-	16	16	498		
2053	-	-	16	16	514		
2054	-	-	16	16	530		
2055	-	-	16	16	546		
2056	-	-	16	16	562		
Total	189	130	243	562			

Note(s):

1. Rockfill placement for 2023 (complete) and 2024 (ongoing).



4.0 QUANTITATIVE PERFORMANCE PARAMETERS (OPERATIONS)

4.1 GENERAL

QPPs provide quantitative parameters that are indicators of performance that can be easily measured and evaluated on-site without complex calculation or data interpretation. QPPs are a good reference to quickly assess the performance and development of the YDTI and RDS. The QPPs can be measured and monitored during facility development to confirm construction is meeting the specifications. Active QPPs are formally presented in the TOMS Manual. The QPPs are regularly reviewed and revised when required as part of annual updates to the TOMS Manual. QPP status is monitored regularly as part of the surveillance activities during ongoing operations at the mine.

4.2 PRELIMINARY QUANTITATIVE PERFORMANCE PARAMETERS

Preliminary geometric QPPs for the two main stages of the proposed YDTI embankment development (i.e. the EL. 6,500 and EL. 6,560 ft crest elevations) and the RDS around the facility are summarized in Table 4.1. These QPPs were developed to guide the embankment layout geometry and construction sequencing prepared for the YDTI and adjacent RDS. The Preliminary QPPs will be reviewed (and revised, if required) during preparation of the IFC designs with the latest versions incorporated into revisions to the TOMS Manual.

Stage	Embankment Limb	Layout QPP ¹	Value
	Namela Oassella	Overall Downstream Slope	No steeper than 2.5H:1V
	North-South	Minimum Crest Width ¹	> 200 ft
EL. 6,500 ft		Overall Downstream Slope	No steeper than 2.5H:1V
Embankment	East-West	Downstream Upper Slope ²	No steeper than 2H:1V
Crest		Minimum Crest Width ¹	> 200 ft
	Moot	Downstream Overall Slope	No steeper than 3H:1V
	West	Minimum Crest Width ^{1,4}	> 200 ft
	North-South	Overall Downstream Slope	No steeper than 3H:1V
	North-South	Minimum Crest Width ¹	> 200 ft
EL. 6,560 ft		Overall Downstream Slope	No steeper than 3H:1V
Embankment	East-West	Downstream Upper Slope ³	No steeper than 3H:1V
Crest		Minimum Crest Width ¹	> 200 ft
	\\/aat	Overall Downstream Slope	No steeper than 3H:1V
	West	Minimum Crest Width ^{1,4}	> 200 ft
Rock Disposal	ΔII	Overall Downstream Slope	No steeper than 3H:1V

Overall Downstream Slope

Table 4.1 Preliminary Geometric QPPs – Operations

Note(s):

Sites

- 1. Minimum crest width considers embankment portion only (excludes substantial upstream surcharge zone).
- 2. Upper slope measured from the Seep 10 bench to embankment crest.

ΑII

- 3. Upper slope measured from the booster pump station 3 bench to embankment crest.
- 4. West Embankment includes Zones D1 and U (embankment).
- 5. H:V = horizontal : vertical (H:V).



No steeper than 3H:1V

5.0 FACILITY LAYOUT AND CONSTRUCTION CRITERIA

5.1 GENERAL

The life of mine development for the YDTI is presented as a phased sequence to illustrate major milestones during the proposed remaining life of mine. The phased designs of the embankment and RDS were developed considering the layout and construction criteria described below. These criteria are based on historical construction practices and layout methodology with the objective to continuously improve safety of the facility through slope flattening and progressive buttressing of the facility embankments. The proposed phases are described in detail in Section 6 and are summarized as:

- Phase 1: Ongoing construction activities to be completed prior to the permit amendment (Prior to Permit)
- Phase 2: EL. 6,500 ft Embankment Crest Raise
- Phase 3: EL. 6,500 ft Lower Embankment Lifts
- Phase 4: EL. 6,500 ft Embankment Lifts and RDS Expansions
- Phase 5: EL. 6.560 ft Embankment Crest Raise
- Phase 6: Final Life of Mine (Prior to Closure)

5.2 EMBANKMENT DEVELOPMENT

5.2.1 GENERAL

General layout and construction criteria that apply to each embankment limb are summarized below. Criteria specific to each embankment limb and stage are described in subsequent sections. Sections showing typical details (i.e. lift height, slope angle, and bench width) to achieve 2.5H:1V and 3H:1V overall embankment slopes during construction are shown on Figure 5.1.

- Embankment Zone U upstream slopes are specified at angle of repose or flatter in select areas to facilitate placement of Zone F materials along the upstream face of each lift. 'F' material will be placed as required with a slope of 2H:1V or flatter to maintain a separation zone between the tailings and embankment rockfill (Zone U).
- Overall embankment downstream slopes are specified and will be achieved by incorporating benches
 between successive 50 to 100 ft high angle of repose slopes (consistent with historical construction
 practices). The width and frequency of the benches controls the overall slope angle, and wider benches
 than used historically will need to be incorporated to create flatter overall slope angles to align with the
 geometric QPPs for each stage.
- Overall embankment crest width may vary for each limb and crest elevation. The embankment layout considers previously constructed surfaces and designs, requirements for future raises, and priority areas of downstream construction based on rockfill availability.
- Regrading, shaping and reclamation of any final downstream slopes should be completed concurrently where and when practicable.



5.2.2 NORTH-SOUTH EMBANKMENT

The North-South Embankment will primarily comprise Zone U constructed in maximum 50 ft thick lifts using the downstream embankment construction method. Typical sections for the North-South Embankment at EL. 6,500 ft and EL. 6,560 ft are shown on Figure 5.2. Key layout criteria used for the development of the embankment design were as follows:

Setting Out line

• The SOL is generally aligned with the upstream edge of the crest (or interface between the surcharge and embankment zones, where present). The position of the SOL changes for each stage (i.e. changes for the 6,500 ft and 6,560 ft crest elevations).

Crest Width

- The minimum embankment crest width is 220 ft measured perpendicular from the SOL towards the downstream side of the embankment.
 - The crest between Section 0+00 and north of Section 8+00 N is wider than the minimum due to the upstream surcharge zone.
 - The North RDS will be progressively constructed downstream of the northeast trending embankment limb which provides additional buttressing and will significantly increase the crest width of that area in later phases.

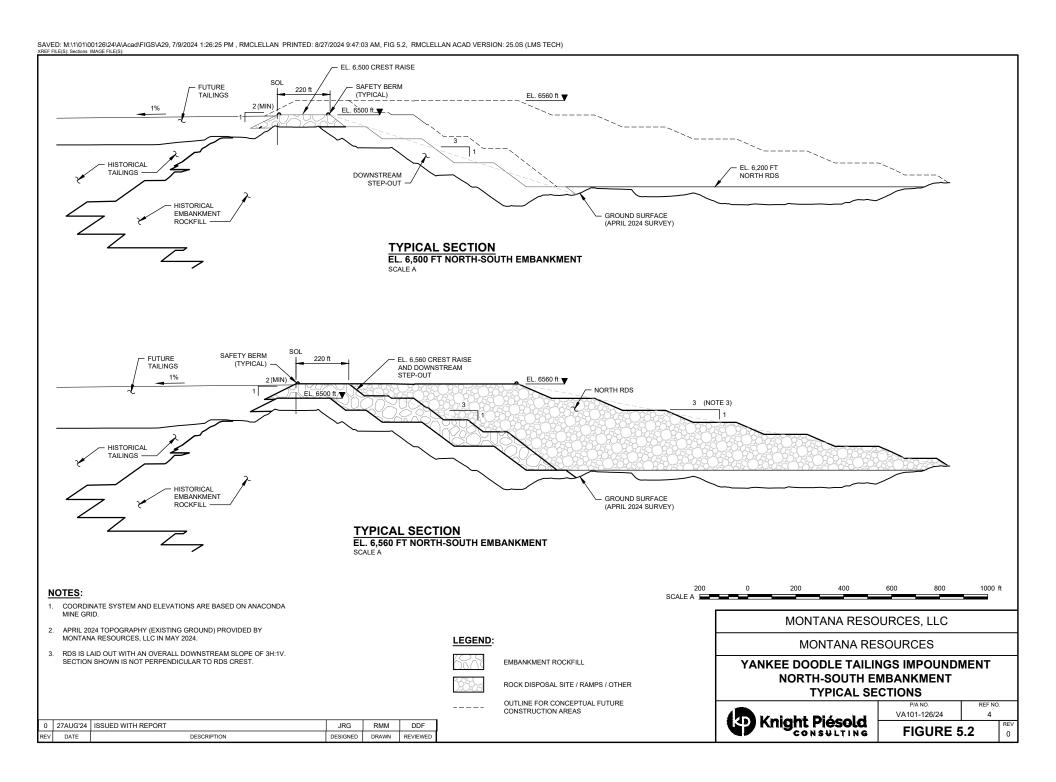
Downstream Slope

 The minimum overall downstream slope is approximately 3H:1V following the EL. 6,500 crest raise (Phase 2) and EL. 6,560 crest raise (Phase 5). The North-South Embankment will be progressively buttressed by the North RDS resulting in flatter overall slope angles during buttress development.

Upstream Slope

The design incorporates a final upstream slope for each lift of approximately 2H:1V or flatter following
placement of Zone F to create a continuous zone connecting to the Zone F layer along the previous lift.





5.2.3 EAST-WEST EMBANKMENT

The East-West Embankment will primarily comprise Zone U constructed in a maximum 50 ft thick lifts using centerline and downstream embankment construction methods. Typical sections of the East-West Embankment at EL. 6,500 ft and EL. 6,560 ft are shown on Figure 5.3. Key layout criteria used for the development of the embankment design are as follows:

Setting Out line

• The SOL is generally aligned with the upstream edge of the structural portion of the crest (interface between the surcharge and embankment zones). The position of the SOL changes for each stage (i.e. changes for the 6,500 ft and 6,560 ft crest elevations). The proposed EL. 6,500 ft raise design follows centreline construction methods, and the EL. 6,560 embankment raise design follows the downstream method.

Crest Width

 The minimum crest width is 220 ft measured perpendicular from the downstream edge of the crest towards the SOL. Most of the embankment crest width is significantly wider due to upstream transition and rockfill surcharge loading zones at EL. 6,500 and EL. 6,560 ft.

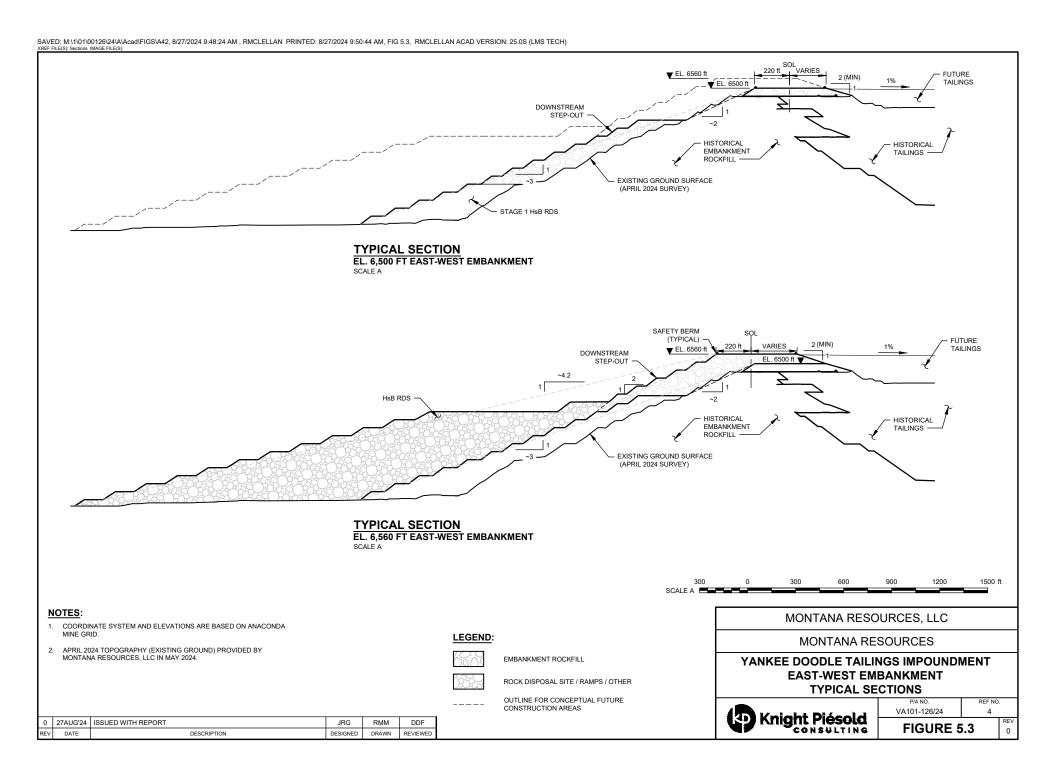
Downstream Slope

- Overall downstream slope of approximately 3H:1V or flatter from the downstream side EL. 6,500 ft crest
 raise to the downstream toe of the Stage 1 HsB RDS. The downstream slope above the Stage 1 HsB
 RDS (i.e. the upper slope) is approximately 2H:1V and will be progressively flattened to approximately
 3H:1V as the lower embankment lifts for the downstream step-out are constructed.
- Overall downstream slope of approximately 4H:1V following the EL. 6,560 ft crest raise and considering
 the full HsB RDS development. The downstream slope of the upper slope (i.e. from the relocated
 tailings pipeline ramp to the downstream edge of the crest) is approximately 2H:1V.

Upstream Slope

 The design incorporates a final upstream slope for each lift of approximately 2H:1V or flatter following placement of Zone F to create a continuous zone connecting to the Zone F layer along the previous lift.





5.2.4 WEST EMBANKMENT

The EL. 6,450 ft West Embankment was constructed with a substantial width to accommodate future raises up to EL. 6,560 ft without significant disturbance beyond the existing downstream toe. Future embankment raises will be completed using the centerline method over the existing wider base with the final layout achieving a configuration like a downstream constructed embankment. The embankment fill will comprise primarily Zone U south of Section 63+00 NW, and a combination of Zone U and thinner lifts in Zone D1 north of Section 63+00 NW. The 'U' and 'D1' materials will be placed in maximum 50 ft and 5 ft thick lifts, respectively. Typical sections of West Embankment at EL. 6,500 ft and EL. 6,560 ft are shown on Figure 5.4. Key layout criteria used for the development of the embankment design are as follows:

Setting Out line

• SOL is aligned 70 ft from the downstream edge of the EL. 6,500 ft and EL. 6,560 ft embankment crest and is the approximate interface between Zone D1 and Zone U at the crest, where Zone D1 is present.

Crest Width

- The minimum embankment crest width is 220 ft measured perpendicular from the downstream edge to the upstream edge of the crest at EL. 6,560 ft.
- The EL. 6,500 ft lift crest width will be significantly wider than the minimum 220 ft because of the construction approach for this embankment.

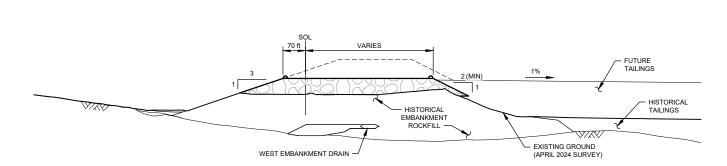
Downstream Slope

- The EL. 6,500 ft lift has been laid out with an overall downstream slope of approximately 3H:1V, except between approximately Section 50+00 NW and 65+00 NW where the downstream slope will be slightly steeper (approximately 2H:1V) to maintain the existing WED Extraction Pond. This is consistent with the current slopes for the EL. 6,450 ft crest.
- The EL. 6,560 ft lift has been laid out with an overall downstream slope of approximately 3H:1V, except at the following locations:
 - The area immediately upslope of the Extraction Pond, as described above, and
 - North of approximately Section 110+00 NW, which will gradually become slightly steeper moving to the north (up to approximately 2.1H:1V) to limit further disturbance on the downstream side beyond the existing EL. 6,450 ft embankment toe.

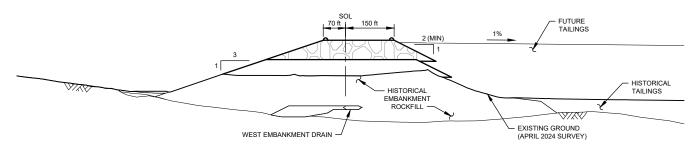
Upstream Slope

The design incorporates a final upstream slope for each lift of approximately 2H:1V or flatter following
placement of Zone F to create a continuous zone connecting to the Zone F layer along the previous lift.





TYPICAL SECTION EL. 6,500 WEST EMBANKMENT SCALE A



TYPICAL SECTION EL. 6,560 FT WEST EMBANKMENT SCALE A

NOTES:

- COORDINATE SYSTEM AND ELEVATIONS ARE BASED ON ANACONDA MINE GRID.
- APRIL 2024 TOPOGRAPHY (EXISTING GROUND) PROVIDED BY MONTANA RESOURCES, LLC IN MAY 2024.

LEGEND:



EMBANKMENT ROCKFILL

OUTLINE FOR CONCEPTUAL FUTURE CONSTRUCTION AREAS

MONTANA RESOURCES, LLC

MONTANA RESOURCES

YANKEE DOODLE TAILINGS IMPOUNDMENT
WEST EMBANKMENT
TYPICAL SECTIONS



VA101-126/24	4	
FIGURE 5	5.4	REV 0

750 ft

REF NO.

0	27AUG'24	224 ISSUED WITH REPORT		RMM	DDF
REV	DATE	DESCRIPTION	DESIGNED	DRAWN	REVIEWED

5.3 ROCK DISPOSAL SITE LAYOUTS

Three areas (two existing and one new) located immediately downstream of the YDTI embankments are proposed for use as RDS for surplus rockfill from mine operations. The RDS will continue to enhance the stability of the YDTI by providing a buttressing effect in select areas and may provide opportunities for progressive reclamation at the mine site once the final footprints are established.

The RDS will be constructed by the mine fleet in maximum 50 ft thick lifts and developed progressively as surplus rockfill is available from mining. The overall downstream slopes specified will be achieved by incorporating benches between successive 50 ft high angle of repose slopes. Excess rockfill above the capacity of the proposed RDS locations is expected to be stored within the Continental Pit following mine operations; however, storage of rockfill within the pit is not a component of the Design Document as it is not related to YDTI development.

A description and key layout criteria for the three RDS are as follows:

HsB (East-West) RDS:

- Located within the HsB area and will be completed over the life of the mine. Stage 1 of the RDS
 (to EL. 5,900 ft) is underway and anticipated to be constructed prior to the crest raise to EL. 6,500 ft.
 Stage 1 provides storage for approximately 20 Mtons of rockfill.
- Progressive HsB RDS construction throughout the remaining mine life will further enhance stability of
 the East-West Embankment in the central pedestal area and may be required concurrently with
 construction of the relocated tailings pipeline ramp to maintain target factors of safety for embankment
 stability (KP, 2024a).
- Expansion of the HsB RDS above EL. 5,900 ft requires the relocation of the existing high voltage transmission line along the EL. 5,900 ft bench.
- Future expansion of the HsB RDS south of the Stage 1 RDS requires the relocation of the truck shop and maintenance yard.
- Overall downstream slopes are specified at 3H:1V to enhance surface reclamation potential.
- Provides approximate rockfill storage capacity of 130 Mtons in addition to the Stage 1 HsB RDS capacity.

North RDS:

- Located downstream of the North-South Embankment and initially permitted under Amendment 10.
- Construction of the EL. 6,200 ft lift is anticipated to be constructed prior to the crest raise to EL. 6,500 ft.
- Overall downstream slopes are specified at 3H:1V.
- Provides approximate rockfill storage capacity of 75 Mtons above EL. 6,200 ft.
- The North RDS will also include storage for alluvium for use during reclamation.

West RDS:

- Located along the northwest trending limb of the East-West Embankment in the area formerly known as the Northwest Dumps.
- Overall downstream slopes are specified at 3H:1V to enhance reclamation potential along this section
 of the East-West Embankment.
- Provides approximate rockfill storage capacity of 30 Mtons.



5.4 EMBANKMENT AND RDS DRAINAGE CONSIDERATIONS

The West Embankment IFC designs may include provisions for the extension of the secondary seepage collection systems associated with the WED as required to manage runoff from the upstream catchment(s). The secondary seepage collection drains will be designed in a similar manner as previous designs and will follow the topographic lows below the embankment rockfill. The purpose of the secondary seepage collection drains is to direct any runoff and seepage through the embankment materials to the WED to prevent water accumulation along the downstream toe of the embankment. The secondary drains will be extended as required prior to each embankment raise.

The alignment for the long-term outlet from the WED Extraction Pond has been considered in the layout of the East-West Embankment and West RDS. This alignment will allow overflow from the pond via a piped outlet to be passively conveyed to the HsB area following closure.

The HsB RDS will include considerations for new drainage features and expansion of the Stage 1 HsB RDS Drainage System (KP, 2023a). Drainage features within the HsB area will be specified as part of the detailed design for the HsB RDS expansion, and IFC drawings may include:

- Conversion or replacement of Surface Water Ditch (SWD) #10 along the Seep 10 bench to a rock drain.
- Conversion or replacement of SWDs #8 and #9 around the truck shop and maintenance yard to rock drains.
- Additional surface water and collection ditches around the RDS if required.
- Additional rock drains at various locations within the RDS foundation.



6.0 PROPOSED FACILITY DEVELOPMENT

6.1 GENERAL

Construction of the embankments and RDS will be completed as a continuous activity as rockfill is available from mine operations. The delivery of embankment construction materials will be scheduled to coincide with availability of rockfill from the mine to meet the phased lift construction requirements. The timing required for the completion of each phase will depend on rockfill availability, tailings production rates, variability of the tailings density throughout the facility, final beach slopes, and the supernatant pond area and volume. The filling of the YDTI will be monitored throughout operations, and construction timing will be adjusted as required. A simplified lift schedule is shown on Figure 6.1, which is based on the filling schedule included in Appendix A, and the phased construction sequence is described in the sections that follow.

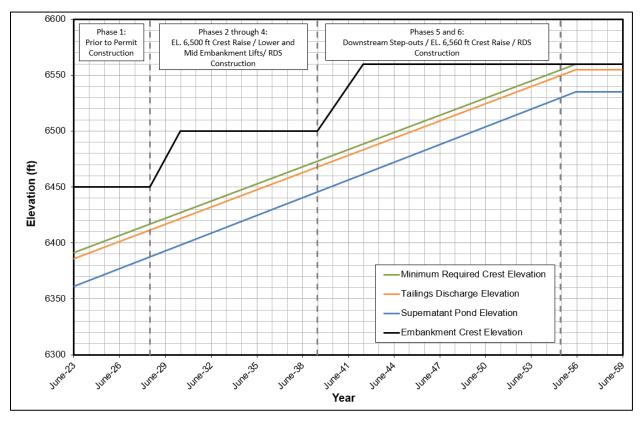


Figure 6.1 YDTI Embankment Lift Schedule

The development of the YDTI relies on various supporting activities to be undertaken by MR (or others) prior to select construction activities. This includes the following:

- Relocation of the high-voltage transmission line along the EL. 5,900 ft elevation bench (Seep 10 bench) of the East-West Embankment prior to construction of the lower embankment lifts in this area.
- Relocation of the truck shop and maintenance yard for continued HsB RDS development after Stage
 1.



• Relocation of Tailings Booster Station No. 3 to facilitate raising of the embankment from EL. 6,500 ft to EL. 6,560 ft.

The timing of these supporting activities is described within the associated development phases below.

IFC designs for the various development stages will be progressively issued during ongoing mine development. The YDTI development sequence presented in this report will be updated as required for the IFC designs, and activities attributed to the phases in this report may be adjusted or combined in the future as part of the ongoing facility development process.

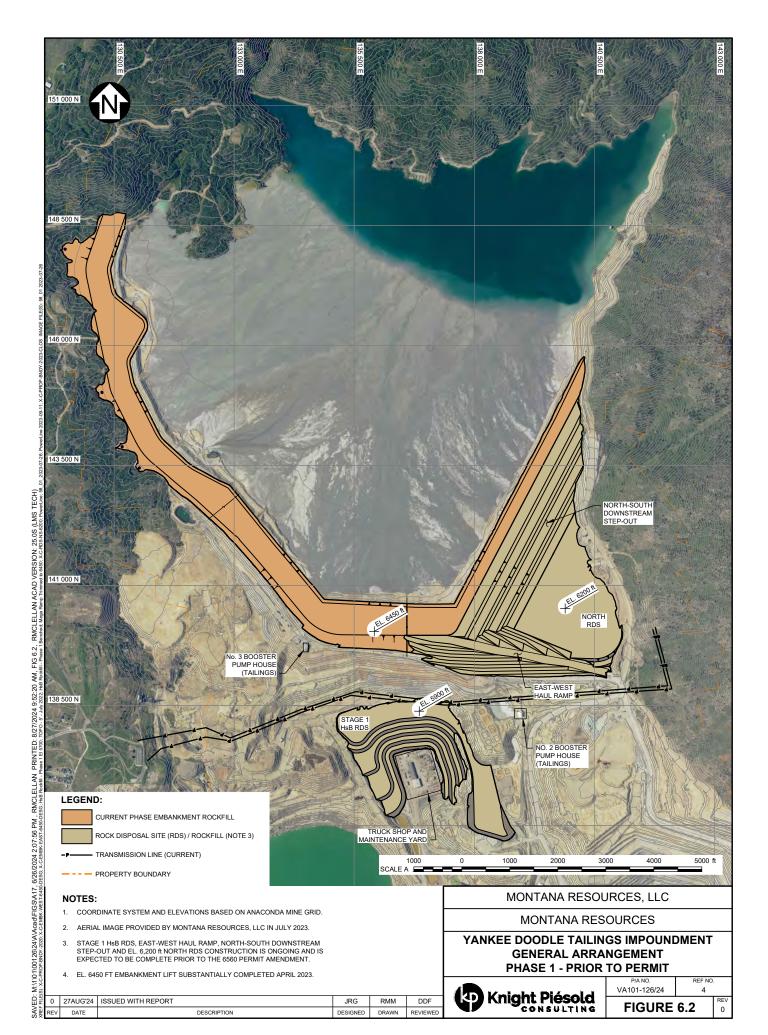
6.2 PHASE 1: PRIOR TO PERMIT

Phase 1 construction outlines the expected areas of construction that will be progressively developed below EL. 6,450 ft (currently permitted crest elevation) over the next several years prior to the requirement for storage expansion. This arrangement, shown on Figure 6.2, provides the base for future construction activities following the permit amendment. Construction activities assumed to be completed prior to the crest raise to EL. 6,500 ft include:

- Construction of the Stage 1 HsB drainage system and the Stage 1 HsB RDS to approximately EL. 5,900 ft.
- Construction of the new East-West Haul ramp to EL. 6,450 ft.
- Construction of downstream lifts of the North-South Embankment to achieve a downstream slope of approximately 3H:1V for the EL. 6,500 ft embankment crest raise.
- Construction of the North RDS along the downstream toe of the existing North-South Embankment to approximately EL. 6,200 ft.

Construction in these areas is ongoing at the time of this report. Construction practices are being completed as per the Amendment 10 YDTI CMP (KP, 2018), the HsB RDS CMP (KP, 2023b) and the KP letter outlining the construction practices for the RDS (KP, 2023c). MR will continue to develop these areas as rockfill is available, with the expectation that the permit amendment will be received with sufficient time to allow for raising of the YDTI embankments to EL. 6,500 ft by the early 2030s.





6.3 PHASE 2: EL. 6,500 FT EMBANKMENT CREST RAISE

Phase 2 is the completion of the embankment crest raise to EL. 6,500 ft as shown on Figure 6.3. This phase will be completed by placement of a single 50 ft lift above the existing EL. 6,450 ft embankment crest and downstream lifts of the North-South Embankment completed during Phase 1.

The YDTI embankment geometry following this construction phase is summarized as follows:

- The downstream toe of the Stage 1 HsB RDS is set out with a projected slope of approximately 3H:1V from the crest of East-West Embankment when measured at Section 0+00. The Stage 1 HsB RDS has a downstream slope of 3H:1V and is assumed to be completed to enhance embankment stability prior to the crest raise. The upper portion of the East-West Embankment will have a downstream slope of approximately 2H:1V, when measured at Section 0+00 from the downstream edge of the embankment crest to the Seep 10 bench.
- The West Embankment will maintain an overall downstream slope of 3H:1V with a fixed downstream toe position aligned with the existing EL. 6,450 ft embankment. Foundation preparation will be required where the embankment alignment extends to the north.
- The North-South Embankment has an overall downstream slope of approximately 3H:1V in this phase, as measured at Section 33+00N.

The Phase 2 layout is estimated to be required for continued tailings storage by late 2034. The current rockfill schedule indicates sufficient material will be available to complete construction of this phase by approximately 2030 assuming construction commences in 2028. This crest elevation and arrangement provides tailings storage capacity through approximately 2044.

6.4 PHASE 3: EL. 6,500 FT LOWER EMBANKMENT LIFTS

Phase 3 includes rockfill placement along the downstream side of the East-West Embankment and modification of the mine haul ramp system to reach EL. 6,500 ft along the North-South Embankment. The Phase 3 activities may commence while Phase 2 is still in progress; however, relocation of a high-voltage transmission line along the Seep 10 bench is required prior to continuing construction of the downstream lifts of the East-West Embankment above the Seep 10 bench. Construction activities associated with Phase 3 are shown on Figure 6.4 and do not result in increased tailings storage for the facility. Construction activities during this phase are focused on continued slope flattening along the East-West Embankment and facility development in preparation for future raises. Activities during this phase include:

- Progressive construction of embankment lifts above the Seep 10 Bench and Stage 1 HsB RDS on the
 downstream side of the East-West Embankment. The lifts are set out to maintain a downstream slope
 of 3H:1V or flatter and will buttress the lower embankment slopes above the Seep 10 bench while
 avoiding the 7% ramp and HsB Capture System Booster Pump House #2. Embankment construction
 in this area requires the following preparatory activities:
 - Conversion of the SWD #10 to a rock drain prior to fill placement.
 - The relocation of the existing high-voltage transmission lines running along the Seep 10 bench.
 Preliminary routing, as determined by Northwest Energy, is included on the general arrangement.
- Widening of the EL. 6,335 ft bench downstream of the Number 3 Tailings Booster Pump House to provide an area for future relocation of the pump house if/when required (expected to be required for raising the embankment crest to EL. 6,560 ft).



- Construction of a new tailings pipeline ramp along the downstream side of the embankment. Relocation
 of the tailings pipelines onto this new pipeline ramp will be required prior to advancing lifts of the
 East-West Embankment above the existing pipeline ramp.
- Construction of a haul ramp along the North-South Embankment to EL. 6,500 ft to provide access from the East-West Haul Ramp to the embankment crest along the North-South Embankment.

Phase 3 construction activities are expected to occur between approximately 2030 and 2036 based on the estimated volumes and current rockfill mining schedule. Drawings presenting the combined embankment design for EL. 6,500 ft through Phase 3 are included as Appendix B.

Further expansion of the HsB RDS outside of the Stage 1 footprint may be required during development of the Phase 3 works to further enhance stability along the downstream toe of the embankment. The extent of additional buttressing will be determined during the IFC phase based on final design geometry for the downstream embankment lifts and new tailings pipeline ramp. Relocation of the truck maintenance workshop and updates to the drainage system design in the HsB area will be required before the HsB RDS can be expanded.

6.5 PHASE 4: EL. 6,500 FT EMBANKMENT LIFTS AND RDS EXPANSIONS

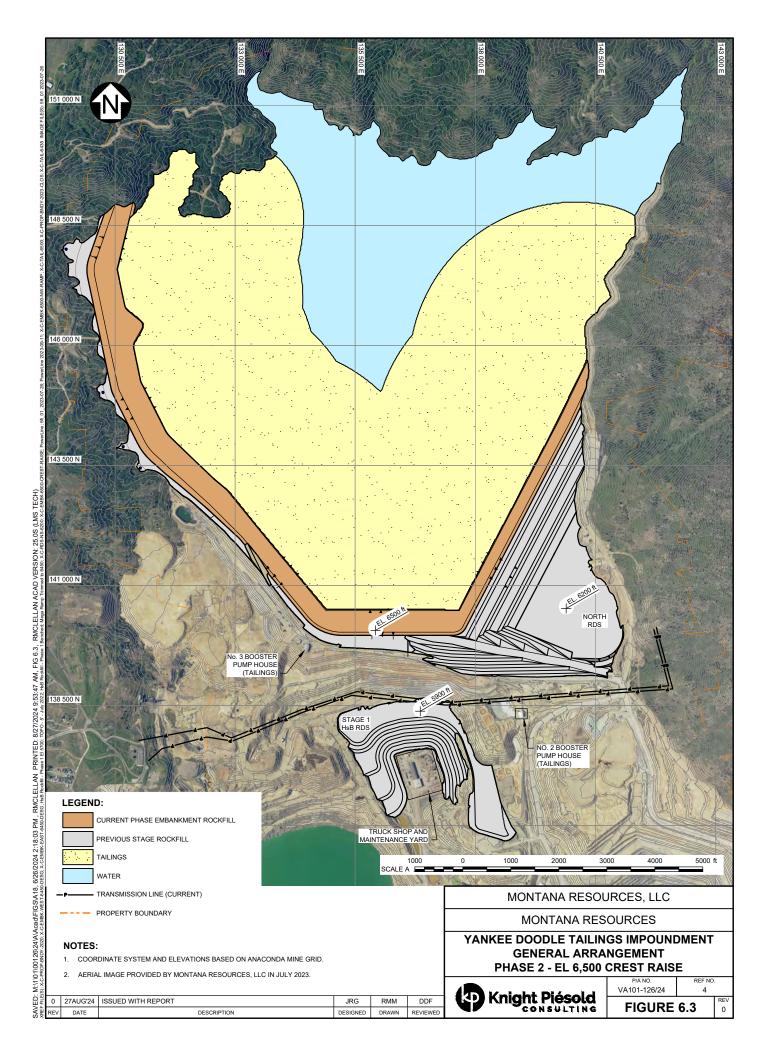
The Phase 4 construction activities described in the following section may commence while Phase 2 or 3 construction is still in progress. These activities are shown separately for illustration purposes in this report but may be adjusted or combined in the future. Phase 4 includes rockfill placement downstream of the East-West and North-South Embankments up to EL. 6,500 ft, construction of a haul ramp along the northwest trending limb of the East-West Embankment, and expansion of the HsB and North RDS as shown on Figure 6.5. Following completion of this phase the downstream step-out of EL. 6,500 ft embankment crest will be fully constructed in anticipation of any future crest raises. Subsequent lifts up to EL. 6,560 ft will be completed as capping lifts to increase storage capacity (i.e., no additional downstream step-outs are required).

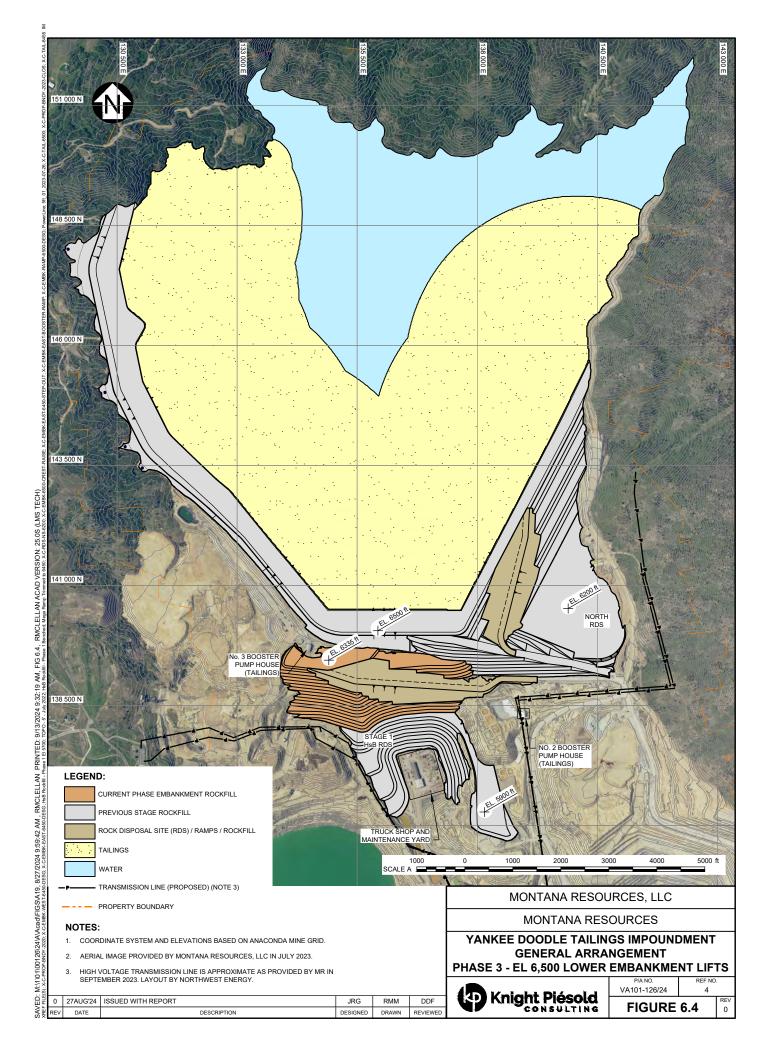
Continued expansion of the North RDS to EL. 6,350 ft and the HsB RDS to EL. 6,200 will flatten the overall downstream slope of the embankments in these areas and provide additional buttressing to the YDTI. The maintenance yard and truck shop located within the HsB area will be relocated prior infilling of the U-shape of the Stage 1 HsB RDS.

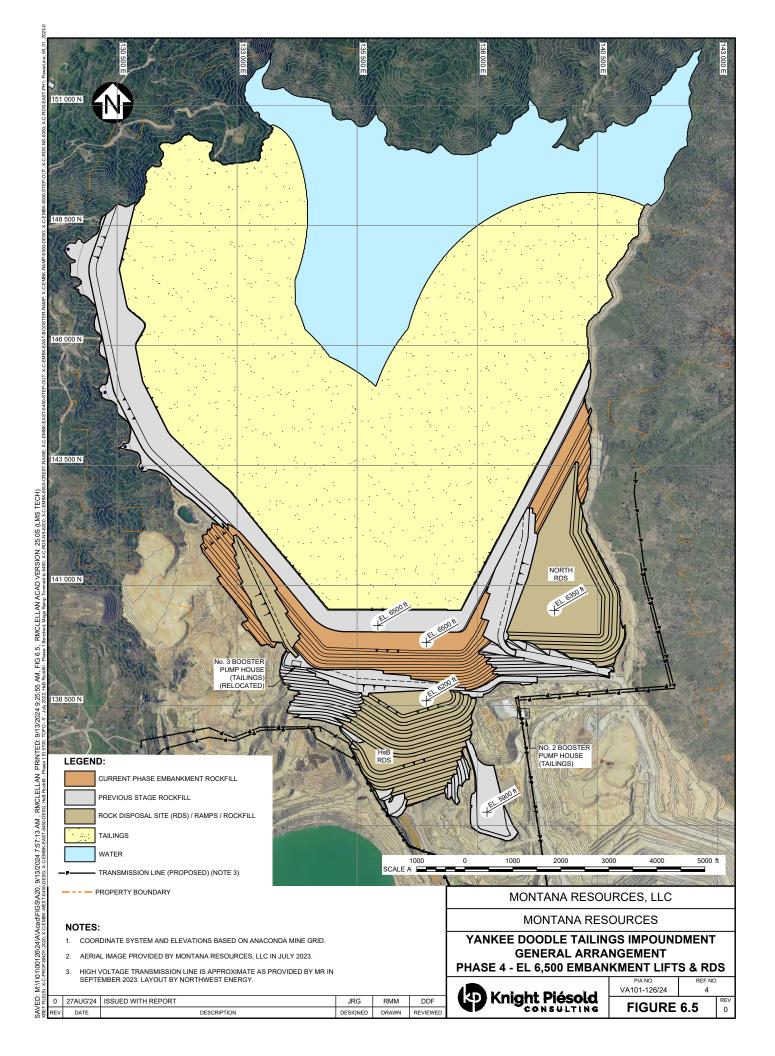
The overall slope of the East-West Embankment from the toe of the HsB RDS to the downstream side of the embankment, measured at Section 0+00, will be approximately 4H:1V. The downstream slope of the upper portion of the East-West Embankment, measured at Section 8+00W, will be approximately 2H:1V between the toe of the mid-embankment lifts and the downstream side of the embankment crest. The overall downstream slope of the North-South Embankment and the North RDS will be 3H:1V.

This construction phase is projected to be complete by approximately 2039 based on the current rockfill mining schedule, to allow for construction of embankment lifts associated with Phase 5 prior to the requirement for additional tailings storage in 2044.









6.6 PHASE 5: EL. 6,560 FT EMBANKMENT CREST RAISE

Phase 5 is the completion of the embankment crest raising up to a maximum of EL. 6,560 ft (excluding the tailings distribution corridor, which may be locally higher to facilitate tailings distribution to the northern ends of the embankment), as shown on Figure 6.6. The maximum allowable lift thickness will be 50 ft; therefore, a minimum of two lifts will be required to reach EL. 6,560 ft. The crest raise will be completed with capping lifts over the previously constructed embankment, and do not require any additional downstream construction to meet target slope angles outlined in the QPPs.

The overall downstream slope of the East-West Embankment will be approximately 3H:1V when measured at Section 0+00 from the toe of the HsB RDS to the downstream side of the embankment crest. The upper portion of the East-West Embankment has an overall downstream slope of 2H:1V, when measured at Section 0+00 from the upstream edge of the pipeline ramp to the downstream side embankment crest. The West Embankment and North-South Embankment will maintain an overall downstream slope of 3H:1V.

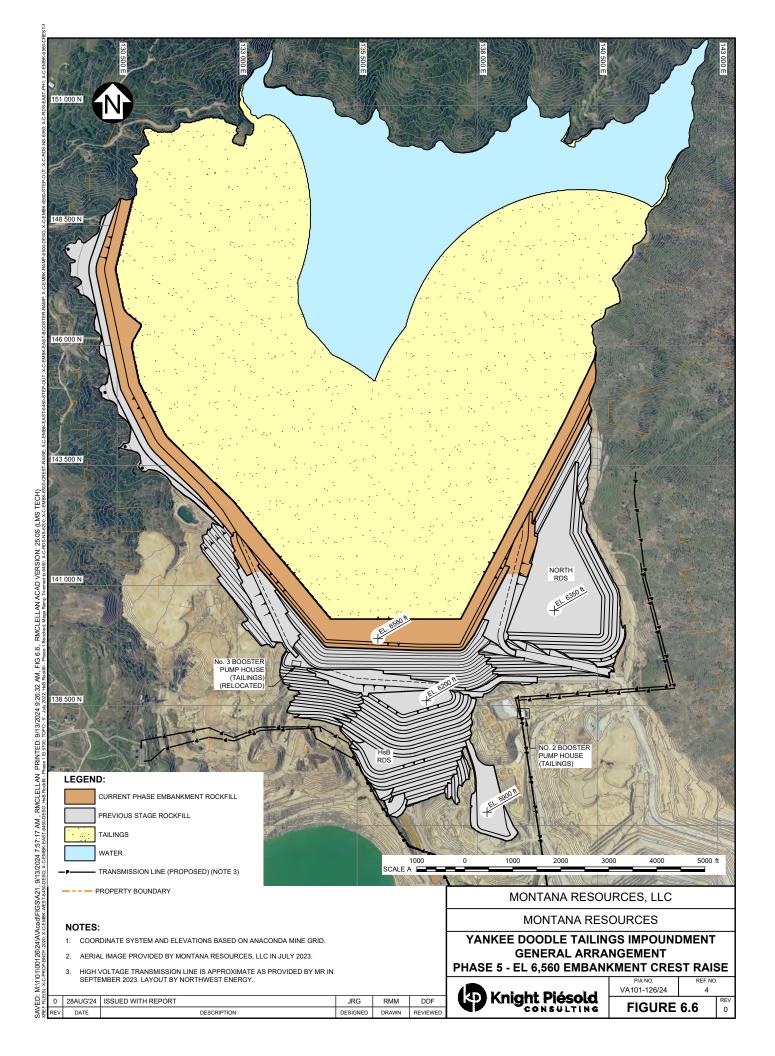
A crest raise above EL. 6,500 ft is estimated to be required for continued tailings storage by approximately 2044. The current rockfill schedule projects sufficient material will be available to complete this phase of construction to meet the storage requirements.

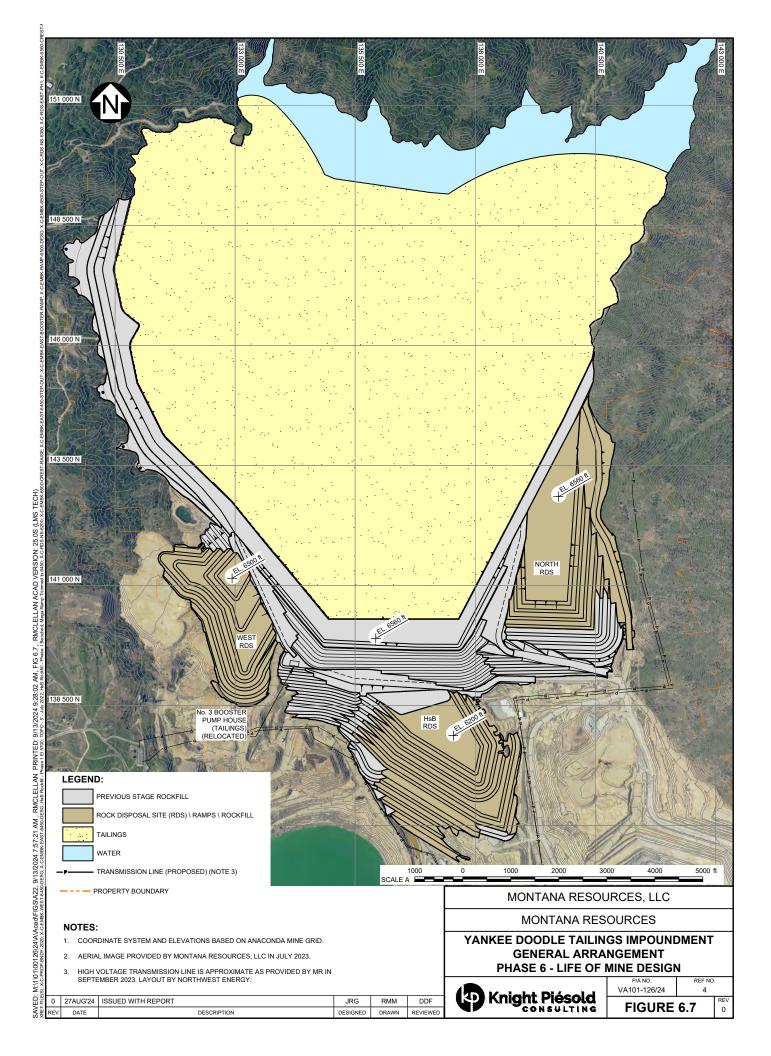
6.7 PHASE 6: LIFE OF MINE DESIGN (PRIOR TO CLOSURE)

The YDTI and supporting RDS will reach full build-out during Phase 6, shown on Figure 6.7, and reclamation and closure activities will be underway as practicable to transition the facility to achieve end land-use objectives. Design drawings presenting the life of mine design are included in Appendix C.

Rockfill placement in this phase is focused on the continued expansion of the HsB RDS, North RDS and development of the West RDS. The HsB RDS footprint is expanded at EL. 6,200 ft while additional lifts are added to the North RDS to reach an ultimate elevation of 6,560 ft. The West RDS is expanded to a maximum of EL. 6,500 ft, while maintaining access to the embankment crest from the access road parallel to the East-West Embankment. Continued expansions of the RDS, with minimum overall downstream slopes of 3H:1V, will result in a significantly flatter overall downstream slopes when measured from the downstream edge of the embankment to the toe of the RDS.







7.0 INSTRUMENTATION AND MONITORING

7.1 GENERAL

KP and MR have progressively advanced characterization of the YDTI embankments, tailings mass and foundation materials, while developing a robust instrumentation network and associated monitoring programs. An initial 5-year phased site investigation and instrumentation plan was developed as part of the Amendment 10 permit application for continued use of the YDTI facilitated by continued construction of the embankment to a crest elevation of 6,450 ft (KP, 2017). The majority of the initial 5-year plan was implemented through a series of site investigation programs completed from 2017 through 2021. Modifications were made based on findings of preceding investigations, in response to feedback from the Independent Review Panel (IRP), and/or to achieve updated project objectives.

An updated 5-year site investigation, instrumentation, and monitoring plan for the YDTI covering the period of 2022 through 2026 was developed in late 2021 (KP, 2021). The objectives of the updated 5-year site investigation, instrumentation and monitoring plan include expanding spatial coverage of subsurface investigations from 2022 through 2026 to:

- Advance characterization of the nature and distribution of rockfill and foundation materials within the East-West and North-South Embankments.
- Further investigate rockfill saturation within the embankments including the influence of rockfill material properties and distribution on drainage within the embankment.
- Install additional pore water pressure and surface/subsurface deformation monitoring instrumentation to progressively supplement the operational instrumentation network within the East-West and North-South Embankments.

7.2 EXISTING MONITORING NETWORK

The YDTI monitoring network includes the active instrumentation to monitor piezometric conditions, displacements within the embankments, tailings, and foundation materials and seepage discharge. Available techniques include:

- Nested vibrating wire piezometers (VWPs) installed at over 60 locations to monitor embankment, tailings, and foundation pore water pressures and at 10 and 28 locations within the Horseshoe Bend and West Ridge areas, respectively.
- Instrumented and manually surveyed inclinometers to monitor subsurface deformations within embankment and foundation materials.
- Elexon Geo4Sight instrumentation (multi-node pore water pressure and angular deformation instruments) to monitor pore water pressures and subsurface angular deformations within tailings and embankment rockfill.
- Global Navigation Satellite System (GNSS) instrumented and manually surveyed survey-monuments to monitor embankment surface displacements.
- Geophysical casings to facilitate borehole nuclear magnetic resonance testing.
- Two seismic switches installed at Booster 3 and the Berkeley Garage to monitor for seismic event driven ground movements in proximity to the YDTI.



 Ultra-sonic seepage monitoring instruments to monitor seepage discharge to Seep 10 and the HsB area from the YDTI.

The active YDTI monitoring program also utilizes interferometric synthetic aperture radar (inSAR) remote sensing to monitor surface deformations throughout the embankments. These programs run during the snow-free season (approximately mid-April through November) and include Long-term, high resolution inSAR monitoring (SqueeSAR; 2-dimensional Terra-SAR-X) with reports typically completed monthly during the snow free season. Additional short-term, frequent inSAR monitoring (Bulletins; 1-dimensional Ascending Terra-SAR-X) with reports completed every 11-days and each covering a monitoring period of 22-days have been used previously during construction and may be reactivated during active YDTI construction phases.

The majority of instrumentation data are available to KP and MR via a remote monitoring system (RMS) that was installed in 2017 and has been progressively expanded thereafter. VWP, GNSS, and instrumented inclinometer data are available in near real time via the online Sensemetrics platform. InSAR data are not included in the system but are reported on by the provider (TRE-Altamira) and analyzed by KP. Elexon Geo4Sight instrumentation are incompatible with Sensemetrics and are downloaded by MR and analyzed by KP external to the RMS. The RMS will continue to operate during the next 5-year period (and beyond) and new instrumentation will be added following installation (as applicable).

7.3 EXPANSION OF MONITORING NETWORK

Maintenance and expansion of the instrumentation and monitoring network around the YDTI and within the RDS will continue throughout the development of the facility. The expansion of the network will initially use similar methods to those described above. It is anticipated that the available technology and state of practice will continue to evolve over the long design life presented in this report, and the methods used for monitoring will evolve to remain consistent with applicable, appropriate and current technologies and techniques practicable for the conditions at the mine.

Specific locations for ongoing monitoring, including pore pressures and deformation behavior (surface and subsurface), will be developed and updated as construction progresses. The practice of developing and following site investigation, instrumentation, and monitoring plans for the YDTI at regular intervals will continue during the development phases described in this report. It is anticipated that additional instrumentation will be installed within the embankments and RDS following completion of specific phases to limit the potential for damage or loss of instrumentation during construction. The instrumentation and monitoring plans, QPPs, and response plans will be progressively updated based on facility performance.



8.0 CLOSURE PLAN

8.1 GENERAL

Detailed information on the reclamation plan and closure of the facility is outlined in the Preliminary Closure Plan prepared by WESTECH Environmental Services, Inc. (WESTECH, 2024). A brief summary of the closure plan is included below for reference.

One of the principal design objectives for ongoing YDTI development was to progressively improve the surface reclamation potential of the YDTI and surrounding facilities. The design presented in this report achieves this objective through gradual slope flattening of overall slope angles and progressive buttressing of the facility embankments. These progressive improvements are made possible by continued mining at the site.

Most embankment and RDS slopes are proposed to be developed with a benched arrangement at overall slope angles of approximately 3H:1V, except in few areas where incorporating these flatter slopes was not practicable. Overall embankment downstream slopes are specified and will be achieved by incorporating benches between successive 50 to 100 ft high angle of repose slopes. Angle of repose slopes would be graded to 3H:1V and intervening benches would be obliterated during reclamation (or progressively during mine operations once final footprints are set) to create the final slope arrangement for closure. The crest, benches, and final slopes will be covered by soil and revegetated as specified in the closure plan. Drainage features will be incorporated to control runoff and keep water from ponding.

The tailings beaches are not expected to require substantial regrading following closure except where undesirable settlement is observed. Selective tailings distribution near the end of operations may be used to develop some topographic diversity. The upland beach areas will be capped and vegetated as specified in the closure plan. Transition zones developing as the water inventory in the closure pond is drawn down from approximately 15,000 acre-feet (ac-ft) to 5,000 ac-ft can be progressively reclaimed during active closure. The water inventory in the final closure pond is expected to continue to reduce below 5,000 ac-ft thereafter but may experience some seasonal and interannual fluctuation depending on prevailing climate conditions in the long-term following closure. A wetland area is anticipated to form near the edge of the final closure pond with wetland plans naturally establishing over time.

The closure arrangement incorporates a closure spillway to be constructed following cessation of operations to integrate a closure plan that enhances dam safety. The objective of the closure spillway is an emergency water management system designed primarily to prevent overtopping of the embankment, but which also serves to prevent pooling of water adjacent to the embankment if severe flooding were to occur. The spillway configuration is presented at a conceptual level, with further detail and evaluation required to develop the detailed design and the construction strategy. The final design details will depend on the final elevation and configuration of the facility at closure.

Water management at the site in the long-term following closure is anticipated to be part of the Butte Mine Flooding Operable Unit (BMFOU) remedy. Any water retention at the YDTI contemplated as part of the BMFOU long-term strategy for post-closure water management should be reviewed and approved by the YDTI Engineer of Record (EOR) unless the facility is declassified and an EOR is determine not to be required.



8.2 MONITORING, INSPECTION AND REVIEW REQUIREMENTS

8.2.1 GENERAL

A detailed closure monitoring plan identifying site-specific needs for monitoring, inspection and review will be prepared prior to closure. The plan will include details such as the monitoring requirements, monitoring frequency and minimum qualifications of monitoring personnel. The plan will include a section describing monitoring and mitigation measures for response to unusual occurrences or emergency conditions.

The reclaimed YDTI facilities initially will be inspected and maintained regularly following closure to verify that any changes to site conditions and facility performance do not compromise the integrity and safety of the impoundment. The performance of the reclamation aspects not directly related to dam safety (i.e. dust generation, revegetation coverage, and surface drainage etc.) is under the jurisdiction of the Montana Department of Environmental Quality (MDEQ) or Environmental Protection Agency (EPA) and are not included in this conceptual plan.

8.2.2 QUANTITATIVE PERFORMANCE PARAMETERS (CLOSURE)

Preliminary QPPs for closure for the end of mine life layout are summarized in Table 8.1. QPPs will be monitored and revised based on the performance of the facility and will be integrated into the final closure monitoring plan for the facility. Updates may be required in the future depending on the final geometry of the facility at closure.

Stage **Embankment Limb Layout QPP** Value Downstream Overall Slope No steeper than 3H:1V North-South > 200 ft Minimum Crest Width Downstream Overall Slope No steeper than 3H:1V **Embankment** East-West Downstream Upper Slope¹ No steeper than 2H:1V (at Closure) Minimum Crest Width > 200 ft No steeper than 3H:1V Downstream Overall Slope West Minimum Crest Width > 200 ft Volume (Active Closure)2 < 18,000 acre-ft Supernatant Pond Volume (Passive Closure)2 < 5,000 acre-ft No ponded water within 800 ft **Tailings Beach** Minimum Beach Length³ of the embankment crest Geometry Consistent with specifications Spillway Condition and Integrity Potential flow not obstructed Rock Disposal ΑII Downstream Overall Slope No steeper than 3H:1V Sites

Table 8.1 Preliminary QPPs - Closure

Note(s):

- 1. Upper Slope measured from the relocated tailings pipeline ramp upstream edge to the downstream embankment crest.
- The pond volume requirement is for maximum normal conditions during active and passive closure. Active closure is defined as a period of 20 years following permanent cessation of mine operations, and passive closure considers the long-term closure condition thereafter.
- 3. The minimum beach length of 800 ft allows time to respond and mitigate water approaching the embankment. The spillway should be positioned to passively maintain the minimum beach length criteria.



8.2.3 MONITORING FREQUENCY

The frequency of monitoring for the reclaimed YDTI will vary depending on the final reclamation plan implemented and the required functionality of each facility component. A post-closure monitoring program and schedule will be prepared following completion of the YDTI reclamation program.

The initial monitoring frequency will be higher following reclamation, while the regraded slopes stabilize, tailings consolidate, and vegetation establishes. Monitoring frequencies and reporting protocols will initially follow the latest version of the TOMS Manual with consideration for changes that may be made annually as part of the normal review processes for this document.

Amendment of the monitoring schedule may be considered once closure monitoring baselines have been established and the reclamation system performance is demonstrated. The YDTI monitoring components that may be included in the post-closure monitoring program are summarized in Table 8.2. The routine inspections and monitoring of the reclaimed facility will be undertaken by personnel familiar with the reclamation objectives and expectations.

Table 8.2 Post-Closure Monitoring Considerations

Location	Inspection	Frequency ¹
	Inspect for cracking, slumping/deformation, erosion, slope failure, and any other changes in the embankment shape and tailings surface. Inspect the upstream slope, downstream slope and embankment crest.	Quarterly
Capped Surfaces (Embankment and	Inspect for daylighting seeps on the downstream embankment slope/benches, water pooling/ponding, soft/wet areas	Monthly
Beach)	Inspect beach surface for dusting risk/potential ²	Monthly ²
	Measure water levels in the monitoring wells and piezometers	Monthly
	Reclaimed beach slope/settlement survey ²	Annually ²
	Vegetation survey for noxious weeds, die-off and disease ²	Annually ²
	Measure pond water level ²	Monthly ²
Pond and Wetland Areas	Evaluate pond water storage volume ²	Annually ²
7 11 0 0 0	Inspect wetland plants for noxious weeds, die-off and disease ²	Annually ²
Spillway	Inspect spillway intake, channel base and side slopes for erosion, Spillway blockage, damage, slope failure, and any other changes in the shape and surface of the spillway.	
HsB Seepage Collection System ³	Record the HsB Weir flowrate ³	Monthly ³
West Embankment Drain	West Embankment Record WED flowrate	
Site Wide Water Management	Inspect surface drainage ditches and culverts for erosion, blockage, damage ²	Quarterly and after large storm events ²

Note(s):

- The frequency of monitoring may be reduced once the reclamation system performance has been established and a monitoring baseline has been established.
- 2. Inspection requirements and inspection frequency are under the jurisdiction of the MDEQ, this component has been included in the table for completeness.
- Inspection requirements and inspection frequency are under the jurisdiction of the EPA, this component has been included in the table for completeness.



8.2.4 INSPECTION AND REVIEWS

8.2.4.1 ENGINEER OF RECORD

The EOR role for the YDTI will be retained following closure. The role and required qualifications for the post-closure EOR will be consistent with those of the EOR as defined in MCA 82-4-375. The post-closure EOR will be responsible for reviewing new documents and design features pertaining to the YDTI, conducting annual inspections and notifying the relevant owner and government authorities (e.g. MDEQ and EPA) if the facility is not performing as intended or poses a threat to human health or the environment.

The EOR will conduct an Annual Inspection, which considers both the geotechnical and reclamation performance of the closed facility. The EOR will prepare a report describing the scope of the inspection and actions recommended to document the closed facility is being properly maintained. The EOR will submit the report to the facility owner and MDEQ and immediately notify the MDEQ if the facility presents an imminent threat or when there is the potential for an imminent threat to human health or the environment.

The EOR will conduct Annual Inspections of the YDTI until reclamation is complete and the IRP is convened for a Periodic Review. The frequency of inspections and future involvement of the EOR and IRP thereafter will be discussed during the first Periodic Review following closure and will be incorporated into the detailed closure monitoring plan.

The EOR may be required to conduct additional inspections or monitoring following any unusual event (e.g., earthquake or extreme rainfall event) or as a result of observations made during a routine inspection or monitoring.

8.2.4.2 INDEPENDENT REVIEW PANEL

The IRP will be convened within five years of closure to evaluate the performance of the facility relative to dam safety standards of practice. The frequency of inspections and future involvement of the EOR and IRP thereafter will be discussed during the first Periodic Review following closure and will be incorporated into the detailed post-closure monitoring plan.



9.0 REFERENCES

- Knight Piésold (KP, 2017). Design Basis Report, Rev 2, KP Ref. No. VA101-126/12-1, June 30, 2017. Vancouver, BC.
- Knight Piésold Ltd. (KP, 2018). Yankee Doodle Tailings Impoundment: Construction Management Plan (KP Reference No. VA101-126/12-5 Rev. 3), May 1, 2018.
- Knight Piésold (KP, 2021). 5-Year Site Investigation, Instrumentation and Monitoring Plan for Yankee Doodle Tailings Impoundment. KP Ref. VA21-02063. December 22, 2021. Vancouver, BC.
- Knight Piésold Ltd. (KP, 2023a). Horseshoe Bend Rock Disposal Site Stage 1 Drainage System Report (KP Reference No. VA101-126/25-3 Rev. 0), December 7, 2023.
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- Knight Piésold Ltd. (KP, 2023c). Construction Practices for the rock disposal site at the toe of the North-South Embankment (KP Reference No. VA23-00542), dated May 10, 2023.
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- Montana Resources, LLC. (MR), 2023. Letter to: Jack Standa, Montana Resources LLC. RE: *Montana Resources Ore Reserves as of December 31, 2022.* January 17, 2023.
- Montana Code Annotated (MCA), 2023. *Title 82: Minerals, Oil, and Gas, Chapter 4: Reclamation, Part 3: Metal Mine Reclamation.* Available at: https://leg.mt.gov/bills/mca/title_0820/chapter_0040/part_0030/sections_index.html
- WESTECH Environmental Services, Inc. (WESTECH), 2024. Preliminary Closure Plan Yankee Doodle Tailings Impoundment 6,560 ft Embankment Elevation. Draft. January 17, 2024. Helena, MT.



10.0 CERTIFICATION

This report was	prepared and reviewed by the undersigned.	KNIGHT PIÉSOLD LTD. PERMIT NUMBER — 1001011 — EGBC PERMIT TO PRACTICE
Prepared:	Jason Gillespie, P.Eng. Senior Engineer	
Reviewed:	Daniel Fontaine, P.E. Specialist Engineer Associate	
	YDTI Engineer of Record	
best judgement base on or decisions mad if any, suffered by a	pared by Knight Piésold Ltd. for the account of Montana Reso ed on the information available at the time of preparation. Any de based on it is the responsibility of such third parties. Knight any third party as a result of decisions made or actions based ight not be the most recent revision.	use a third party makes of this report, or any reliance Piésold Ltd. accepts no responsibility for damages,
	Approval that this document adhere	es to the Knight Piésold Quality System:



Montana Resources, LLC Montana Resources Yankee Doodle Tailings Impoundment - Life of Mine Design Report for 6,560 Amendment Design Document

APPENDIX A

Tailings Deposition Modelling and Storage Capacity Evaluation

(Pages A-1 to A-17)







MEMORANDUM

Date: September 3, 2024 **File No.:** VA101-00126/24-A.01

Cont. No.: VA23-02116

To: Mr. Daniel Fontaine
Copy To: Mr. Jason Gillespie

From: Nicanor Villarreal, Roanna Dalton

Re: Yankee Doodle Tailings Impoundment – EL. 6,560 ft Tailings Deposition

Modelling and Storage Capacity Evaluation

1.0 INTRODUCTION

Montana Resources, LLC (MR) is preparing a permit amendment application (6,560 Amendment Application) for continued use of the Yankee Doodle Tailings Impoundment (YDTI). The proposed amendment considers raising the crest elevation (EL.) of the YDTI embankments to a maximum elevation (EL.) of 6,560 ft in two or more lifts to facilitate continued mining until the mid-2050s. This memorandum provides a summary of the tailings deposition modelling and storage capacity evaluation that was undertaken to support the 6560 Amendment Application.

The design of the YDTI references the site coordinate system known as the 'Anaconda Mine Grid' established by The Anaconda Company in 1957. The Anaconda Mine Grid is based on the Anaconda Copper Company (ACC) Datum established in 1915. All elevations are stated in Anaconda Mine Grid coordinates with respect to the ACC Vertical Datum unless specifically indicated otherwise.

The purposes of the tailings deposition modelling and storage capacity evaluation were to:

- Develop a filling schedule for the YDTI to support mine rockfill scheduling for construction of future embankment lifts.
- Develop a conceptual tailings deposition strategy that results in a configuration of the beach and supernatant pond that creates hydraulic connectivity between the water reclaim system, supernatant pond and freshwater inflows from the Yankee Doodle Creek, Dixie Creek, and Silver Bow Creek.

2.0 TAILINGS DEPOSITION MODEL INPUTS

2.1 3D MODEL STAGING AND BASE TOPOGRAPHY

A three-dimensional (3D) tailings deposition model was prepared using the Muk3D computer software package (MineBridge Software Inc., 2023). The software was used to model tailings deposition and beach development using specified tailings volumes, tailings beach slopes, tailings discharge points, and pond volumes.

The model was developed using eight stages of tailings storage within the YDTI. Each stage creates approximately a 20 ft raise of the tailings beach surface. Stage 'Existing' is generally consistent with conditions in July 2023 with an embankment crest elevation of 6,450 ft. Stage 8 is representative of the



facility constructed with an embankment crest elevation of 6,560 ft and filled with tailings to a maximum discharge elevation of 6,555 ft.

The base topographic surface was developed using the 2023 end of year embankment surface provided by MR and the tailings beach topography and pond bathymetry as surveyed in July 2023 (MR, 2023b). The approximate EL. 6,500 ft and EL. 6,560 ft embankment configurations were included as required to increase storage capacity in the model.

2.2 TAILINGS TONNAGE AND PRODUCTION RATE

The 2022 Ore Reserves Report (MR, 2023a) identified approximately 570 million tons (Mt) of proven and probable reserves remaining as of December 31, 2022. The amendment application contemplates providing sufficient storage capacity for at least 570 Mt of tailings corresponding to the remaining reserves.

The mill ore throughput at MR has ranged between approximately 15.6 and 18.4 Mt per year since 2004. The average, minimum and maximum annual estimates of ore throughput at the mine since 2004 are summarized in Table 2.1.

Annual Dry Ore Tons Milled (2004 through 2023)	Throughput (Mt)
Average	17.2
Minimum	15.6
Maximum	18.4

Table 2.1 Historical Ore Production Summary

Approximately 99% of the ore throughput is discharged as tailings from the Concentrator and conveyed to the YDTI. An assumption that ore throughput equals tailings production rate was made for the purpose of the deposition model, and a tailings production rate of 18 Mt per year was assumed for development of the filling schedule. The tailings production rate was chosen to have a representative but conservative value to account for any years that production is near or higher than the maximum recorded throughput.

2.3 TAILINGS CHARACTERISTICS

Seismic Cone Penetration Testing (SCPT) was performed in 2021 to determine the characteristics of consolidated tailings near the embankment crest. Analysis of the SCPT data indicates the settled dry density of the consolidated tailings in the upper 170 ft of tailings beach ranges between 75 pounds per cubic foot (pcf) to 121 pcf, and the mean dry density was estimated to be approximately 98 to 102 pcf.

The deposition model adopted an initial settled dry density of 85 pcf (1.15 tons/yd³) for the tailings deposited in the YDTI. This density is equivalent to a saturated bulk density of approximately 115 pcf. This dry density provides a reasonably conservative estimate of storage capacity and subsequent filling rate for the YDTI. Tailings consolidation was not considered in the modelling undertaken for this study. Long-term consolidation will increase the dry density of the tailings above the initial settled density thereby increasing the available storage capacity of the facility.

The existing beach slopes were assessed using the 2023 aerial imagery and pond bathymetry to establish tailings beach slope criteria for modelling the storage capacity. The estimated beach slope criteria used in the 3D model were as follows:



- Sub-aerial tailings beach 0.5%
- Sub-aqueous tailings beach 4%

2.4 TAILINGS DEPOSITION STRATEGY AND DISCHARGE LOCATIONS

The tailings beach is progressively developed as the tailing slurry is discharged from locations along the YDTI embankments. The tailings solids from the slurry settle and accumulate within the impoundment. The deposition strategy adopted in the tailings deposition model is consistent with the requirement to promote development of extensive drained tailings beaches between the embankment and supernatant pond. The supernatant pond was maintained on the northeast side of the facility in the modelling. The position and shape of the pond is directly controlled by the deposition strategy and the natural topography to the north and east sides of the facility. A nominal operating supernatant pond of 15,000 acre-feet (ac-ft) was considered in each model stage, which is equivalent to the current target nominal operating pond volume for ongoing mine operations.

The tailings discharge spigot plan incorporated in the model consists of discharge points uniformly spaced around the upstream face of the embankments at approximately 500 ft intervals. This spacing is comparable to the spacing implemented around the YDTI in 2023. The spigot plan for the facility was adjusted for the final modelling phase to include targeted tailings deposition in the centre of the facility (the south-west extent of the pond). The infilling is proposed in preparation for closure to promote enhanced hydraulic connectivity between the Yankee Doodle, Dixie and Silver Bow Creeks that discharge into the YDTI from the north and to increase the beach length between the pond and the embankments at closure.

The tailings deposition model applied uniform discharge elevations 5 ft below the embankment crest elevation for all modelling phases. The 5 ft elevation difference was included to provide a practical minimum "dry' operational freeboard allowance between the tailings discharge elevation and the embankment crest elevation for tailings management purposes.

2.5 **MODEL INPUT SUMMARY**

Table 2.2

A summary of the inputs and assumptions for the deposition model is presented in Table 2.2.

Tailings Deposition Model - Summary of Inputs

Criteria	Units	Value
Nominal Milling Rate (Yearly)	Mt/yr	18
Total Tonnage in Reserves ¹	Mt	570
Average Initial Settled Tailings Dry Density	pcf	85
Tailings Beach Slope - Above Water	%	0.5
Tailings Beach Slope - Below Water	%	4.0
Operating Pond Volume	ac-ft	15,000
Minimum Dry Freeboard	ft	5

Note(s):

Estimated reserves tonnage as of December 31, 2022 (MR, 2023a).



3.0 DEPOSITION MODEL RESULTS

3.1 GENERAL

The eight stages of YDTI filling evaluated in the deposition model are the following:

- Two stages with YDTI constructed to EL. 6,450 ft (currently constructed)
- Three stages with YDTI constructed to EL. 6,500 ft (proposed)
- Three stages with YDTI constructed to EL. 6,560 ft (proposed)

The elevations of the embankment crest, tailings discharge and the pond elevation at the end of each of the eight modelling stages are summarized in Table 3.1. Figures demonstrating the deposition modelling results for each of the stages are included in Appendix A.

Stage	Permit	Embankment Crest Elevation (ft)	Tailing Discharge Elevation (ft)	Pond Elevation (ft)
Existing	Existing	6,450	6,395	6,360
1	Existing	6,450	6,415	6,398
2	Existing	6,450	6,435	6,418
3	6,560 ft Amendment	6,500	6,455	6,437
4	6,560 ft Amendment	6,500	6,475	6,456
5	6,560 ft Amendment	6,500	6,495	6,476
6	6,560 ft Amendment	6,560	6,515	6,496
7	6,560 ft Amendment	6,560	6,535	6,516
8	6,560 ft Amendment	6,560	6,555	6,535

Table 3.1 Deposition Model – Stage Summary

3.2 ELEVATION-AREA-CAPACITY CURVES

The YDTI elevation-area-capacity curves developed from the tailings deposition model results are shown on Figure 3.1. The curves show the relationships between the elevation of the tailings and the capacity and surface area of the facility. The capacity curve incorporates the estimated 880 Mt (775 million cubic yards (Myd³)) of historical tailings previously placed in the facility between 1963 and June 2023.

The tailings deposition model and curves indicate that approximately 235 Myd³ of tailings and water storage capacity exists above the July 2023 tailings surface with the YDTI constructed to El. 6,450 ft and filled with tailings to a discharge elevation of 6,445 ft. Continued construction of the YDTI to an embankment crest of EL. 6,560 ft and filling to a discharge elevation of 6,555 ft results in additional tailings and water storage capacity of approximately 350 Myd³. The total estimated capacity of the YDTI constructed to El. 6,560 ft would therefore be approximately 1,350 Myd³.



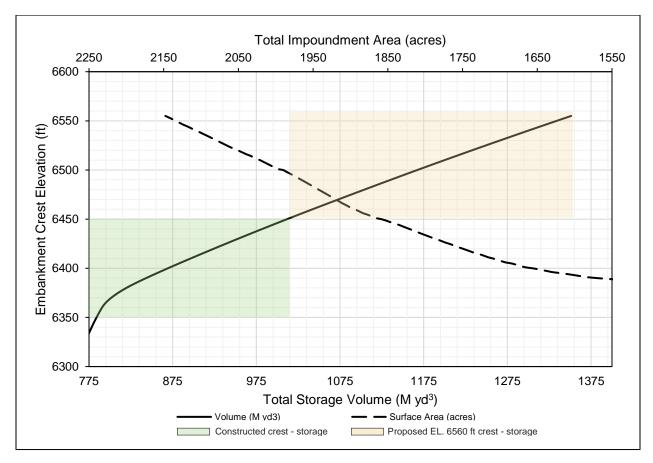


Figure 3.1 YDTI Elevation-Area-Capacity Curves

3.3 TAILINGS FILLING SCHEDULE

The results of the tailings deposition modelling were used to develop the estimated YDTI tailings filling schedule and filling curve as shown in Table 3.2 and Figure 3.2, respectively. The tailings filling schedule presents the modelled tailings storage volume for each of the eight deposition stages, the cumulative storage volume of the facility and an estimate of the approximate year the stage of filling is complete. The tailings storage capacity in the first deposition stage is higher than subsequent stages due to 'infilling' of the 2023 'base' tailings beach surface to establish the 'uniform elevation' discharge spigot configuration for modelling purposes.

The supernatant pond elevation was also estimated from the deposition modelling results. The minimum required crest elevation associated with each filing stage was established based on maintaining a 5 ft minimum "dry" freeboard between the tailings discharge elevation and the crest elevation.



Table 3.2 Filling Schedule

Stage	Discharge Elevation	Minimum Required Crest Elevation ¹	Incremental Tailings Storage (per Stage)	Cumulative Tailings Storage	Pond Elevation	Approx. Year ²
	(ft)	(ft)	(Mt)	(Mt)	(ft)	
Existing	6,386	6,391	0	0	6,361	2023
1	6,415	6,420	107³	107	6,398	2029
2	6,435	6,440	64	171	6,418	2033
3	6,455	6,460	65	236	6,437	2036
4	6,475	6,480	69	305	6,456	2040
5	6,495	6,500	72	377	6,476	2044
6	6,515	6,520	71	448	6,496	2048
7	6,535	6,540	75	523	6,516	2052
8	6,555	6,560	80	603	6,535	2056

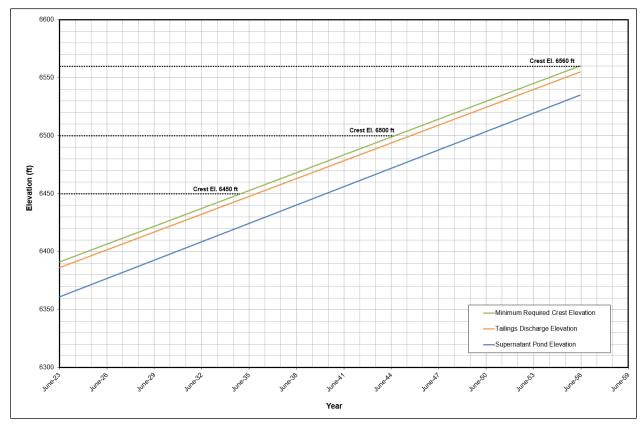
Note(s):

- 1. The minimum required crest elevation is estimated based on a nominal 5 ft dry freeboard between the embankment crest elevation and the discharge elevation of the tailings spigots.
- 2. The calendar year for each modelled filling stage is estimated based on the assumption of 18 Mt of tailings being produced annually.
- 3. The tailings storage capacity in the first deposition stage is higher than subsequent stages due to 'infilling' of the tailings beach surface to establish the 'uniform elevation' discharge spigot configuration for modelling purposes.

The tailings filling curve indicates the existing YDTI constructed with a crest elevation of 6,450 ft will provide tailings storage capacity approximately until approximately December 2034. The YDTI constructed to the proposed crest elevation of 6,560 ft will provide tailings storage capacity until approximately June 2056. The proposed crest raise to EL. 6,560 ft from EL. 6,450 ft therefore provides for approximately 400 Mt of tailings deposition, which is equivalent to approximately 22 years of mine operations.

The rate of rise of the tailings surface is estimated to be approximately 5 ft per year, which is consistent with historical operations at the YDTI. The difference between the tailings discharge and supernatant pond elevations is modelled to be approximately 20 ft during future normal operating conditions. Freeboard during operations would be expected to variable since the discharge elevations at all spigots may not be maintained as uniformly as modelled.





Note(s):

- 1. The tailings discharge elevation rate of rise assumes production of 18 Mt of tailings annually.
- 2. The minimum required crest elevation was estimated based on a nominal 5 ft minimum "dry" freeboard between the embankment crest elevation and the discharge elevation of the tailings spigots.

Figure 3.2 YDTI - Tailings Filling Curve

4.0 EMBANKMENT CREST RAISE SCHEDULE

The embankment crest raise schedule is dictated by the required tailings storage capacity and the availability and timing of rockfill for embankment construction. The results of the tailings deposition model will be used to inform the embankment raise construction schedule.

The following key embankment crest raise milestones were identified based on the results of the tailings deposition model:

- Construction of the EL. 6,500 ft embankment raise is required to be completed by approximately December 2034.
- Construction of the EL. 6,560 ft embankment raise is required to be completed by approximately June 2044.

The proposed schedule and sequencing for the construction of the YDTI, considering the tailings storage capacity and the availability of rockfill, will be presented in subsequent reports.



5.0 CLOSING

The tailings deposition model was developed to support the application process for the 6,560 Amendment Application. The deposition model identified the following:

- 1. The continued construction of the YDTI embankment to raise the crest elevation raise from 6,450 ft to 6,560 ft will provide additional tailings storage capacity of approximately 400 Mt (or approximately 22 years of mine operation).
- 2. The tailings discharge spigot configuration proposed will help development of a supernatant pond shape and position that promotes positive hydraulic connectivity between Yankee Doodle Creek, Dixie Creek, Silver Bow Creek, and the water reclaim pump barges.

The tailings deposition modelling provides an estimate of the tailings discharge and storage management behaviour that is expected to occur during continued use of the YDTI for tailings storage. MR will need to continue to employ the observational approach to monitor the tailings beach development and management of the tailings discharge spigots in order to manage the position of the pond and meet tailings beach development objectives.

We hope this meets your requirements at this time. Please do not hesitate to contact the undersigned with any questions or comments.

Yours truly,

Knight Piésold Ltd.

Prepared: Reviewed:

Project Engineer

Nicanor Villarreal, EIT

Roanna Dalton, P.Eng.

Specialist Engineer | Associate

KNIGHT PIÉSOLD LTD.
PERMIT NUMBER

EGBC PERMIT TO PRACTICE

Approval that this document adheres to the Knight Piésold Quality System:



Attachments:

Appendix A YDTI Tailings Deposition and Capacity Evaluation Modelling Results

References:

MineBridge Software Inc. 2023. Muk3D Mine Modelling Software. Version V2022.1.8

Montana Resources, LLC. (MR), 2023a. Letter to: Jack Standa, Montana Resources LLC. RE: Montana Resources – Ore Reserves as of December 31, 2022. January 17, 2023.

Montana Resources, LLC. (MR), 2023b. Memo to: Daniel Janney, Montana Resources LLC. RE: Bathymetric Survey - 2023. August 8, 2023.

September 3, 2024 8 of 8 VA23-02116

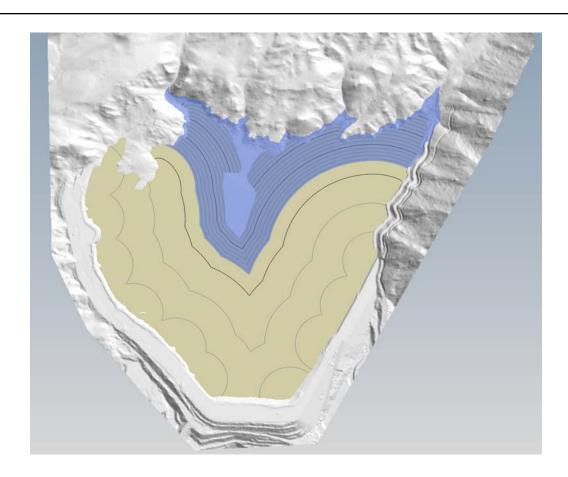


APPENDIX A

YDTI Tailings Deposition and Capacity Evaluation Modelling Results

(Figures A.1 to A.8)

September 3, 2024 VA23-02116



Tailings Inputs	Units	Value
Tailings Discharge Elevation	ft	6,415
Operating Pond Volume	ac-ft	15,000

Tailings Storage Facility Outputs	Units	Value
Tonnage of Tailings Stored Between Discharge at 6,395 ft and 6,415 ft	Mt	107
Cumulative Tonnage of Tailings Stored above 6,395 ft	Mt	107
Embankment Elevation	ft	6,450
Pond Elevation	ft	6,398
Estimated Year Discharge Elevation is Reached	-	2029

1. TAILINGS DEPOSITION MODELLED WITH MUK3D SOFTWARE.

MONTANA RESOURCES, LLC

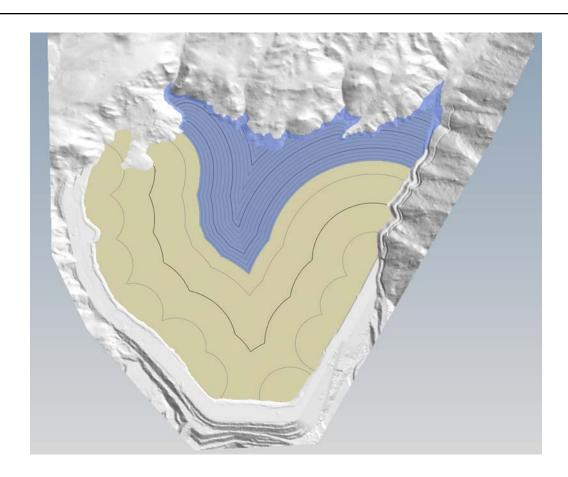
MONTANA RESOURCES

TAILINGS DEPOSITION EVALUATION STAGE 1 EL. 6415 ft TAILINGS DISCHARGE ELEVATION

0	3SEP'24	ISSUED WITH MEMO	CAT	RSD
REV	DATE	DESCRIPTION	PREP'D	RVW'D

(D)	Knight Piésold	VA101-126
47	CONSULTING	FIGU

P/A NO. /A101-126/24 REF. NO. VA23-02116 REF. NO. VA23-02116



Tailings Inputs	Units	Value
Tailings Discharge Elevation	ft	6,435
Operating Pond Volume	ac-ft	15,000

Tailings Storage Facility Outputs	Units	Value
Tonnage of Tailings Stored Between Discharge at 6,415 ft and 6,435 ft	Mt	64
Cumulative Tonnage of Tailings Stored above 6,395 ft	Mt	171
Embankment Elevation	ft	6,450
Pond Elevation	ft	6,418
Estimated Year Discharge Elevation is Reached	-	2033

0

REV

3SEP'24

DATE

1. TAILINGS DEPOSITION MODELLED WITH MUK3D SOFTWARE.

MONTANA RESOURCES, LLC

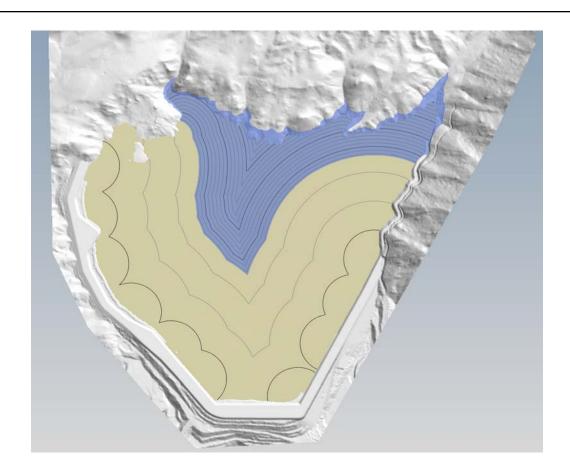
MONTANA RESOURCES

TAILINGS DEPOSITION EVALUATION STAGE 2 EL. 6,435 ft TAILINGS DISCHARGE ELEVATION

ISSUED WITH MEMO	CAT	RSD
DESCRIPTION	PREP'D	RVW'D

1	Knight Piésold	VA101-12
47	Knight Piésold	FIGU

P/A NO. VA101-126/24 REF. NO. VA23-02116



Tailings Inputs	Units	Value
Tailings Discharge Elevation	ft	6,455
Operating Pond Volume	ac-ft	15,000

Tailings Storage Facility Outputs	Units	Value
Tonnage of Tailings Stored Between Discharge at 6,435 ft and 6,455 ft	Mt	65
Cumulative Tonnage of Tailings Stored above 6,395 ft	Mt	236
Embankment Elevation	ft	6,500
Pond Elevation	ft	6,437
Estimated Year Discharge Elevation is Reached	-	2036

0

REV

3SEP'24

DATE

1. TAILINGS DEPOSITION MODELLED WITH MUK3D SOFTWARE.

MONTANA RESOURCES, LLC

MONTANA RESOURCES

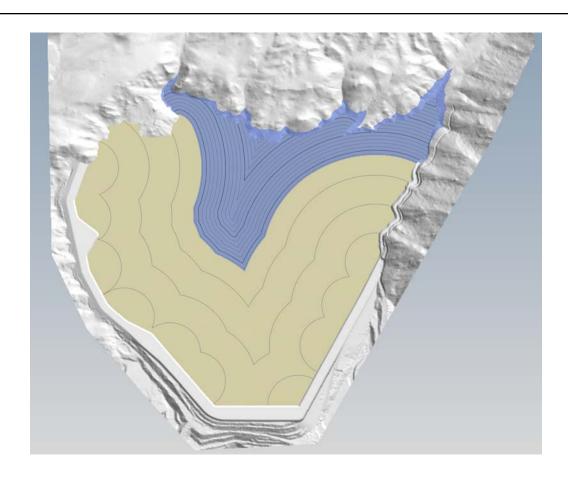
TAILINGS DEPOSITION EVALUATION STAGE 3 EL. 6,455 ft TAILINGS DISCHARGE ELEVATION

ISSUED WITH MEMO	CAT	RSD	
DESCRIPTION	PREP'D	RVW'D	

Might Piésold	,
Knight Piésold	

P/A NO. VA101-126/24 REF. NO. VA23-02116

FIGURE A.3



Tailings Inputs	Units	Value
Tailings Discharge Elevation	ft	6,475
Operating Pond Volume	ac-ft	15,000

Tailings Storage Facility Outputs	Units	Value
Tonnage of Tailings Stored Between Discharge at 6,455 ft and 6,475 ft	Mt	69
Cumulative Tonnage of Tailings Stored above 6,395 ft	Mt	305
Embankment Elevation	ft	6,500
Pond Elevation	ft	6,456
Estimated Year Discharge Elevation is Reached	-	2040

0 REV

1. TAILINGS DEPOSITION MODELLED WITH MUK3D SOFTWARE.

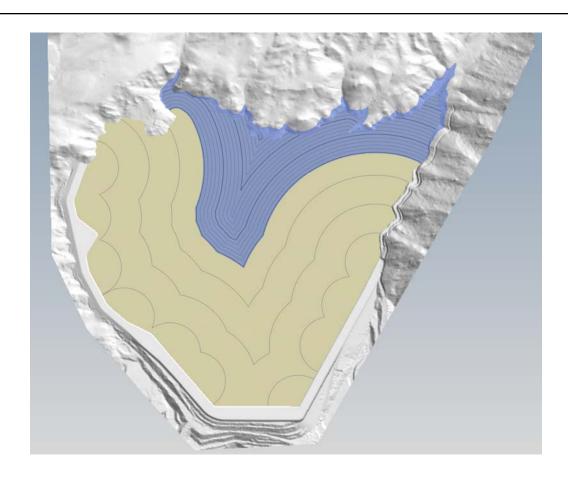
MONTANA RESOURCES, LLC

MONTANA RESOURCES

TAILINGS DEPOSITION EVALUATION STAGE 4 EL. 6,475 ft TAILINGS DISCHARGE ELEVATION

3SEP'24	ISSUED WITH MEMO	CAT	RSD
DATE	DESCRIPTION	PREP'D	RVW'D

Knight Piésold	P/A NO. VA101-126/24	VA23-02	
CONSULTING	FIGURE A	.4	RE'



Tailings Inputs	Units	Value
Tailings Discharge Elevation	ft	6,495
Operating Pond Volume	ac-ft	15,000

Tailings Storage Facility Outputs	Units	Value
Tonnage of Tailings Stored Between Discharge at 6,475 ft and 6,495 ft	Mt	72
Cumulative Tonnage of Tailings Stored above 6,395 ft	Mt	377
Embankment Elevation	ft	6,500
Pond Elevation	ft	6,476
Estimated Year Discharge Elevation is Reached	-	2044

1. TAILINGS DEPOSITION MODELLED WITH MUK3D SOFTWARE.

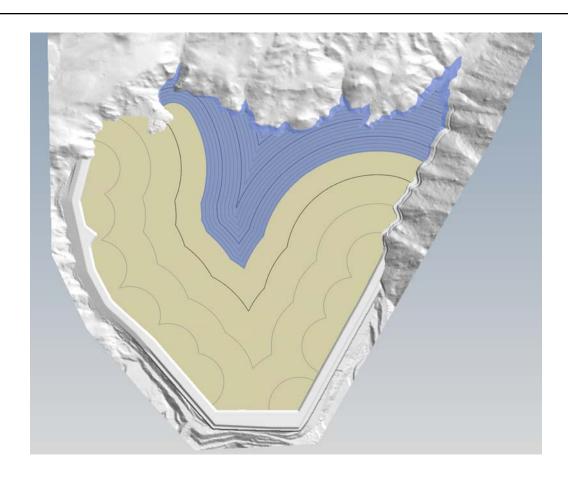
MONTANA RESOURCES, LLC

MONTANA RESOURCES

TAILINGS DEPOSITION EVALUATION STAGE 5 EL. 6,495 ft TAILINGS DISCHARGE ELEVATION

0	3SEP'24	ISSUED WITH MEMO	CAT	RSD
REV	DATE	DESCRIPTION	PREP'D	RVW'D

Knight Piésold	P/A NO. VA101-126/24	REF. NO. VA23-02116	
CONSULTING	FIGURE A	.5	REV 0



Tailings Inputs	Units	Value
Tailings Discharge Elevation	ft	6,515
Operating Pond Volume	ac-ft	15,000

Tailings Storage Facility Outputs	Units	Value
Tonnage of Tailings Stored Between Discharge at 6,495 ft and 6,515 ft	Mt	71
Cumulative Tonnage of Tailings Stored above 6,395 ft	Mt	448
Embankment Elevation	ft	6,560
Pond Elevation	ft	6,496
Estimated Year Discharge Elevation is Reached	-	2048

1. TAILINGS DEPOSITION MODELLED WITH MUK3D SOFTWARE.

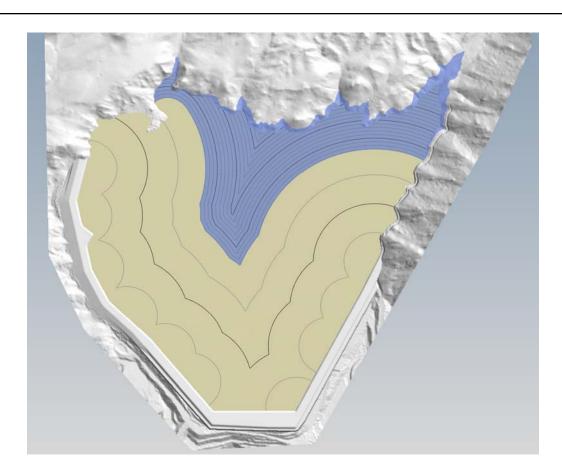
MONTANA RESOURCES, LLC

MONTANA RESOURCES

TAILINGS DEPOSITION EVALUATION STAGE 6 EL. 6,515 ft TAILINGS DISCHARGE ELEVATION

ı					
	0	3SEP'24	ISSUED WITH MEMO	CAT	RSD
	REV	DATE	DESCRIPTION	PREP'D	RVW'D

CONSULTING	FIGURE A	.6	F
Knight Piésold	P/A NO. VA101-126/24	VA23-021	-



Tailings Inputs	Units	Value
Tailings Discharge Elevation	ft	6,535
Operating Pond Volume	ac-ft	15,000

Tailings Storage Facility Outputs	Units	Value
Tonnage of Tailings Stored Between Discharge at 6,515 ft and 6,535 ft	Mt	75
Cumulative Tonnage of Tailings Stored above 6,395 ft	Mt	523
Embankment Elevation	ft	6,560
Pond Elevation	ft	6,516
Estimated Year Discharge Elevation is Reached	-	2052

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REV

1. TAILINGS DEPOSITION MODELLED WITH MUK3D SOFTWARE.

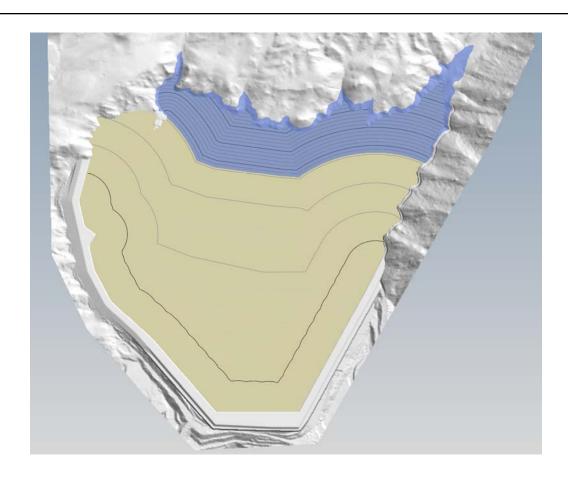
MONTANA RESOURCES, LLC MONTANA RESOURCES

TAILINGS DEPOSITION EVALUATION

STAGE 7
EL. 6,535 ft TAILINGS DISCHARGE ELEVATION

3SEP'24	ISSUED WITH MEMO	CAT	RSD	
DATE	DESCRIPTION	PREP'D	RVW'D	

Knight Piésold	P/A NO. VA101-126/24		
CONSULTING	FIGURE A	.7	REV 0



Tailings Inputs	Units	Value
Tailings Discharge Elevation	ft	6,555
Operating Pond Volume	ac-ft	15,000

Tailings Storage Facility Outputs	Units	Value
Tonnage of Tailings Stored Between Discharge at 6,535 ft and 6,555 ft	Mt	80
Cumulative Tonnage of Tailings Stored above 6,395 ft	Mt	603
Embankment Elevation	ft	6,550
Pond Elevation	ft	6,535
Estimated Year Discharge Elevation is Reached	-	2056

1. TAILINGS DEPOSITION MODELLED WITH MUK3D SOFTWARE.

MONTANA RESOURCES, LLC

MONTANA RESOURCES

TAILINGS DEPOSITION EVALUATION STAGE 8 EL. 6,555 ft TAILINGS DISCHARGE ELEVATION

0	3SEP'24	ISSUED WITH MEMO	CAT	RSD
REV	DATE	DESCRIPTION	PREP'D	RVW'D

Knight Piésold	P/A NO. VA101-126/24	REF. NO. VA23-02116	
CONSULTING	FIGURE A	.8	RE 0

APPENDIX B

Design Drawing Package - 6,500 ft Embankment Crest

Table B.1			
MR-C4011 Rev B	MR-C4310 Rev B	MR-C4336 Rev B	MR-C4443 Rev B
MR-C4020 Rev B	MR-C4311 Rev B	MR-C4337 Rev B	MR-C4444 Rev B
MR-C4110 Rev B	MR-C4330 Rev B	MR-C4338 Rev B	MR-C4445 Rev B
MR-C4111 Rev B	MR-C4331 Rev B	MR-C4410 Rev B	MR-C4446 Rev B
MR-C4129 Rev B	MR-C4332 Rev B	MR-C4411 Rev B	MR-C4447 Rev B
MR-C4130 Rev B	MR-C4333 Rev B	MR-C4440 Rev B	
MR-C4131 Rev B	MR-C4334 Rev B	MR-C4441 Rev B	
MR-C4132 Rev B	MR-C4335 Rev B	MR-C4442 Rev B	





TABLE B.1

MONTANA RESOURCES, LLC MONTANA RESOURCES

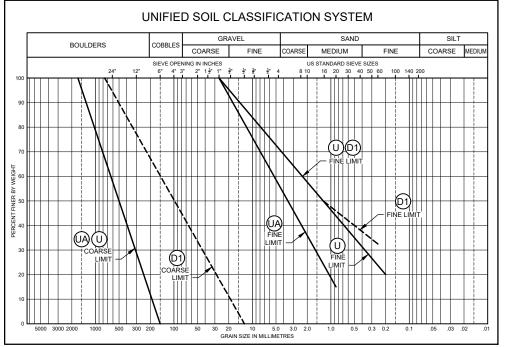
YANKEE DOODLE TAILINGS IMPOUNDMENT LIFE OF MINE DESIGN REPORT 6500 CREST DRAWING LIST

Print Sep/13/24 13:58:38

Drawing Number	Rev.	Revision Date	Drawing Status	Drawing Title
MR-C4011	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - Fill Material Specifications
MR-C4020	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - 6500 Crest - General Arrangement
MR-C4110	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - West Embankment - 6500 Crest - Plan
MR-C4111	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - West Embankment - 6500 Crest - Setting Out Details
MR-C4129	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - West Embankment - 6500 Crest - Sections 53+00 NW and 58+00 NW
MR-C4130	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - West Embankment - 6500 Crest - Sections 60+00 W and 69+50 W
MR-C4131	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - West Embankment - 6500 Crest - Sections 78+50 W and 82+80 W
MR-C4132	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - West Embankment - 6500 Crest - Sections 95+00 W and 108+40 W
MR-C4310	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6500 Crest - Plan
MR-C4311	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6500 Crest - Setting Out Details
MR-C4330	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6500 Crest - Section 0+00
MR-C4331	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6500 Crest - Section 03+00 W
MR-C4332	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6500 Crest - Section 08+00 W
MR-C4333	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6500 Crest - Section 12+00 W
MR-C4334	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6500 Crest - Section 18+00 NW
MR-C4335	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6500 Crest - Section 23+00 NW
MR-C4336	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6500 Crest - Section 28+00 NW
MR-C4337	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6500 Crest - Sections 33+00 NW and 38+00 NW
MR-C4338	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6500 Crest - Sections 43+00 NW and 48+00 NW
MR-C4410	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6500 Crest - Plan
MR-C4411	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6500 Crest - Setting Out Details
MR-C4440	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6500 Crest - Section 3+00 N
MR-C4441	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6500 Crest - Section 08+00 N
MR-C4442	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6500 Crest - Section 13+00 N
MR-C4443	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6500 Crest - Section 18+00 N and 23+00 N
MR-C4444	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6500 Crest - Section 28+00 N and 33+00 N
MR-C4445	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6500 Crest - Section 38+00 N and 43+00 N
MR-C4446	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6500 Crest - Section 48+00 N and 53+00 N
MR-C4447	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6500 Crest - Section 58+00 N and 63+00 N

\knightpiesold.local\VA-Prj\$\1\01\00126\24\A\Report\4 - YDTI Life of Mine Design Report for A6560\Rev 0\Appendices\[App B and C Tables.xlsx]App B

	0	13SEP'24	ISSUED WITH REPORT VA101-126/24-4		DDF
1	REV	DATE	DESCRIPTION	PREP'D	RVW'D

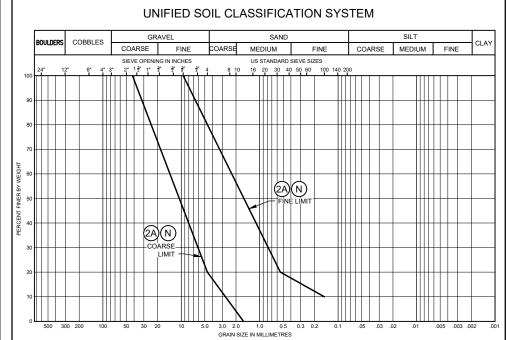


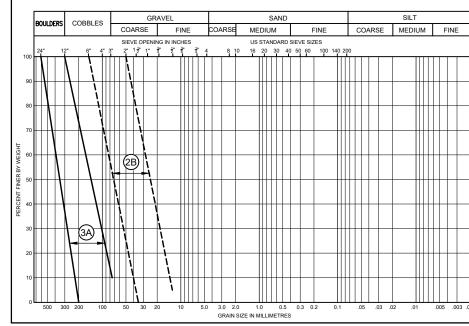
UNIFIED SOIL CLASSIFICATION SYSTEM GRAVEI SAND BOULDERS CLAY COARSE FINE MEDIUM FINE COARSE MEDIUM FINE SIEVE OPENING IN INCHES US STANDARD SIEVE SIZES F @

ZONE U / ZONE D1 / ZONE UA

ZONE F / ZONE D2

UNIFIED SOIL CLASSIFICATION SYSTEM





ZONE N - INSTRUMENTATION BEDDING / **ZONE 2A - FILTER**

REFERENCE DRAWINGS

ZONE 2B - TRANSITION / ZONE 3A - DRAIN ROCK

FOR INFORMATION ONLY **NOT FOR CONSTRUCTION**

DESIGNED	DRAWN	REVIEWED	APPROVED	REV	DATE	DESCRIPTION	DESIGNED	DRAWN	REVIEWED	APPROVED
				Α	27JUN'24	ISSUED FOR CLIENT REVIEW	JRG	RMM	DDF	DDF
				В	27AUG'24	ISSUED FOR PERMITTING	JRG	RMM	DDF	DDF

	MATERIAL PLACEMENT AND COMPACTION REQUIREMENTS
ZONE AND MATERIAL TYPE	PLACING AND COMPACTION REQUIREMENTS
ZONE F - UPSTREAM EARTHFILL	FILL MATERIAL SHALL CONSIST OF ALLUVIUM, FREE OF LOAM, TREE STUMPS, ROOTS, AND OTHER DELETERIOUS OR ORGANIC MATTER. THE MATERIAL SHALL BE END-DUMPED AND SPREAD ALONG THE UPSTREAM SLOPE OF EACH LIFT TO CREATE A CONTINUOUS ZONE CONNECTING TO THE LAYER ALONG THE PREVIOUS LIFT. THE FINAL FILL SURFACE WILL BE SHAPED BY DOZERS WITH THE SURFACE TRACK COMPACTED DURING SPREADING. THE MATERIAL SHALL CONTAIN A BROAD RANGE OF WELL-GRADED SOILS ACROSS THE ENTIRE SPECIFIED GRAIN SIZE ENVELOPE.
ZONE U - UPSTREAM ROCKFILL	FILL MATERIAL SHALL CONSIST OF HARD, DURABLE, FRESH TO MODERATELY WEATHERED ROCKFILL MATERIAL AND SHALL BE END DUMPED IN UP TO 50 ft LIFTS. FILL MATERIAL WILL BE TRAFFIC COMPACTED BY THE MINE HAUL FLEET, EQUALLY DISTRIBUTED OVER THE ENTIRE LAYER WIDTH. THE MATERIAL SHALL CONTAIN A BROAD RANGE OF WELL-GRADED SOILS ACROSS THE ENTIRE SPECIFIED GRAIN SIZE ENVELOPE. COMPACTED RUNNING SURFACES WILL BE CROSS RIPPED PRIOR TO PLACING SUCCESSIVE LIFTS.
ZONE UA - PROTECTIVE CAP	FILL MATERIAL SHALL CONSIST OF HARD, DURABLE, FRESH TO MODERATELY WEATHERED ROCKFILL MATERIAL AND SHALL BE PLACED AND SPREAD IN UP TO 5 ft.LIFTS. FILL MATERIAL WILL BE DOZER COMPACTED. THE MATERIAL SHALL CONTAIN A BROAD RANGE OF WELL-GRADED SOILS ACROSS THE ENTIRE SPECIFIED GRAIN SIZE ENVELOPE. COMPACTED RUNNING SURFACES WILL BE CROSS RIPPED PRIOR TO PLACING SUCCESSIVE LIFTS.
	FILL MATERIAL SHALL CONSIST OF HARD, DURABLE, FRESH TO MODERATELY-WEATHERED ROCKFILL WITH A RELATIVELY LOW ACID GENERATING POTENTIAL. FILL MATERIAL SHALL CONTAIN AS LITTLE POTENTIALLY ACID GENERATING MATERIAL AS PRACTICABLE. THE FILL SHALL BE PLACED IN A MANNER TO PREVENT SEGREGATION. MATERIAL SHALL CONTAIN A BROAD RANGE OF WELL-GRADED SOILS ACROSS THE ENTIRE SPECIFIED GRAIN SIZE ENVELOPE. MATERIAL SHALL BE PLACED AND COMPACTED USING ONE THE FOLLOWING APPROVED METHODS:
ZONE D1 - DOWNSTREAM ROCKFILL	1. MATERIALS SHALL BE PLACED IN MAXIMUM 3 ft LIFTS PRIOR TO COMPACTION. FILL MATERIALS SHALL BE TRAFFIC COMPACTED DURING PLACEMENT WITH 40-ton (CAT740 OR SIMILAR) HAUL FLEET, EQUALLY DISTRIBUTED OVER THE ENTIRE LIFT WIDTH, AND SUBSEQUENTLY COMPACTED USING 6 PASSES OF A SMOOTH DRUM VIBRATORY ROLLER WITH A DRUM WEIGHT OF 12.5 TONNES (13.8 US TONS).
NOON ILL	2. MATERIALS SHALL BE PLACED AND TRAFFIC COMPACTED BY THE 240 TON (CAT 793D) MINE HAUL FLEET IN APPROXIMATELY 5 ft THICK LIFTS, WITH COMPACTION EFFORT EQUALLY DISTRIBUTED OVER THE ENTIRE LIFT WIDTH. COMPACTED MATERIAL LIFTS MAY VARY BY +/- 1 ft THROUGHOUT THE LIFT AREA, PROVIDED THAT LESS THAN 10% OF THE LIFT AREA EXCEEDS THE LIFT TOLERANCE (i.e. IS GREATER THAN 6 ft IN THICKNESS).
	ADDITIONAL METHOD SPECIFICATIONS CAN BE CONSIDERED, INCLUDING DIFFERENT COMPACTION TECHNIQUES OR LIFT THICKNESSES. ANY POTENTIAL REVISIONS TO THE APPROVED METHOD SPECIFICATIONS MAY REQUIRE VERIFICATION BY COMPLETING A TEST FILL AT THE DIRECTION OF THE ENGINEER.
ZONE D2 - DOWNSTREAM EARTHFILL	FILL MATERIAL SHALL CONSIST OF NON-ACID GENERATING ALLUVIUM FREE OF LOAM, TREE STUMPS, ROOTS AND OTHER DELETERIOUS OR ORGANIC MATTER. MATERIAL SHALL BE PLACED AND SPREAD IN 3 ft LIFTS WITH 2 PASSES OF THE SPECIFIED SMOOTH DRUM VIBRATORY ROLLER
ZONE N - INSTRUMENTATION BEDDING	FILL MATERIAL SHALL CONSIST OF HARD, DURABLE, FRESH OR NON-WEATHERED MATERIAL. BEDDING TO BE PLACED AND SPREAD IN 1 ft THICK LIFTS WITH NOMINAL COMPACTION.
ZONE 2A - FILTER MATERIAL	FILTER MATERIAL SHALL CONSIST OF HARD, DURABLE, FRESH OR NON-WEATHERED MATERIAL. FILTER MATERIAL TO BE PLACED AND SPREAD IN MAXIMUM 2 ft THICK LIFTS AND COMPACTED WITH 2 PASSES OF THE SPECIFIED SMOOTH DRUM VIBRATORY ROLLER OR AS DIRECTED BY THE ENGINEER.
ZONE 2B - TRANSITION MATERIAL	TRANSITION MATERIAL SHALL CONSIST OF HARD, DURABLE, FRESH OR NON-WEATHERED MATERIAL. TRANSITION MATERIAL TO BE PLACED AND SPREAD IN MAXIMUM 2 ft THICK LIFTS AND COMPACTED WITH 2 PASSES OF THE SPECIFICED SMOOTH DRUM VIBRATORY ROLLER OR AS DIRECTED BY THE ENGINEER.
ZONE 3A - DRAIN ROCK	DRAIN ROCK SHALL CONSIST OF HARD, DURABLE, FRESH OR NON-WEATHERED ROCK FILL. DRAIN ROCK TO BE PLACED AND SPREAD IN MAXIMUM 3 ft THICK LIFTS AND COMPACTED WITH 2 PASSES OF THE SPECIFIED SMOOTH DRUM VIBRATORY ROLLER OR AS DIRECTED BY THE ENGINEER.

NOTES:

- 1. THESE MATERIAL PLACEMENT AND COMPACTION REQUIREMENTS APPLY TO ALL COMPONENTS OF THE WORKS
- 2. THE MAXIMUM DIMENSION OF ANY PARTICLE SHALL NOT EXCEED 2/3 OF THE MAXIMUM LIFT THICKNESS.
- 3. THIS DRAWING SHALL BE READ WITH ACCOMPANYING DRAWINGS INFORMATION PRESENTED IN THE DESIGN REPORT (KP REF. # VA101-126/24-4)

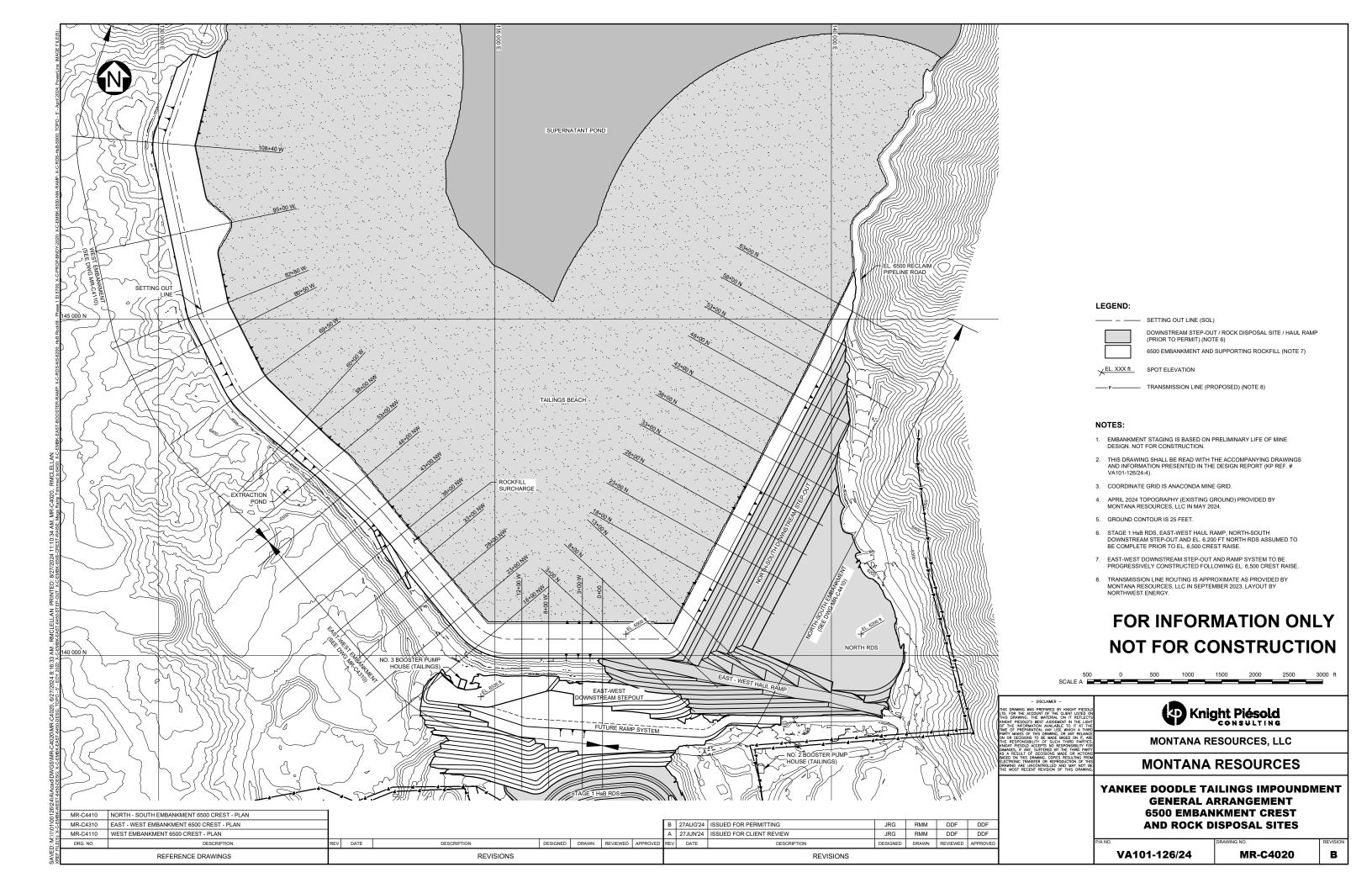
- DISCLAIMER - THIS DRAWING WAS PREPARED BY KNIGHT PIESOLD TID. FOR THE ACCOUNT OF THE CLIENT USTED ON THIS DRAWING, THE MATERIAL ON IT REFLECTS KNIGHT PIESOLD'S BEST JUDGELBERT IN THE LIGHT OF THE INFORMATION AVAILABLE TO IT AT THE TIME OF PREPARATION, ANY USE WHICH A THIRD THEORY THE THEORY OF THE	Knig	ght Piésold	
PARTY MAKES OF THIS DRAWING, OR ANY RELIANCE ON OR DECISIONS TO BE MADE BASED ON IT, ARE THE RESPONSIBILITY OF SUCH THIRD PARTIES. KNIGHT PIESOLD ACCEPTS NO RESPONSIBILITY FOR DAMAGES, IF ANY, SUFFERED BY THE THIRD PARTY	MONTANA RI	ESOURCES, LLC	
AS A RESULT OF DECISIONS MADE OR ACTIONS BASED ON THIS DRAWING. COPIES RESULTING FROM ELECTRONIC TRANSFER OR REPRODUCTION OF THIS DRAWING ARE UNCONTROLLED AND MAY NOT BE THE MOST RECENT REVISION OF THIS DRAWING.	MONTANA	RESOURCES	
		SPECIFICATIONS	
1	P/A NO.	DRAWING NO.	REVISIO

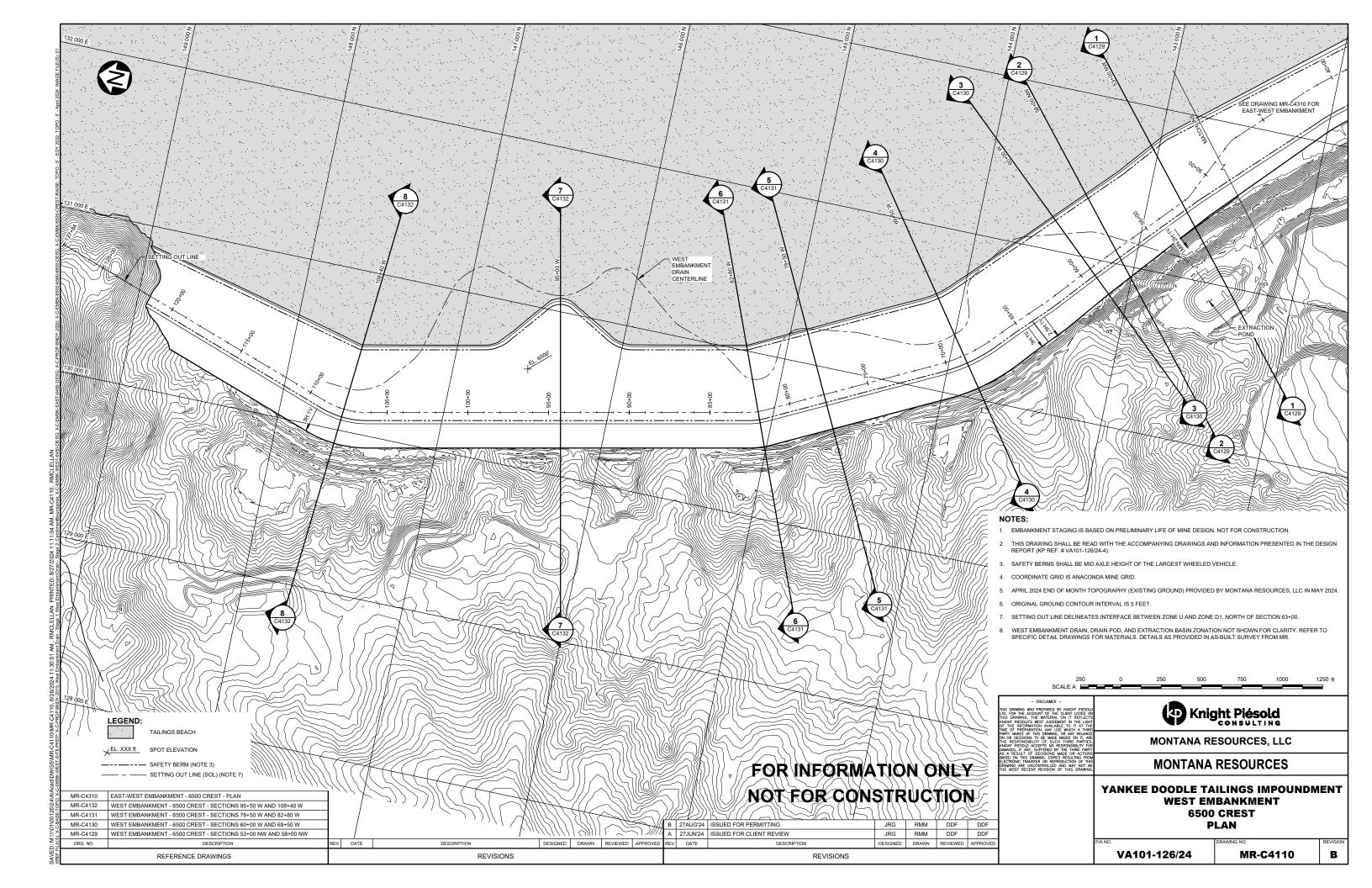
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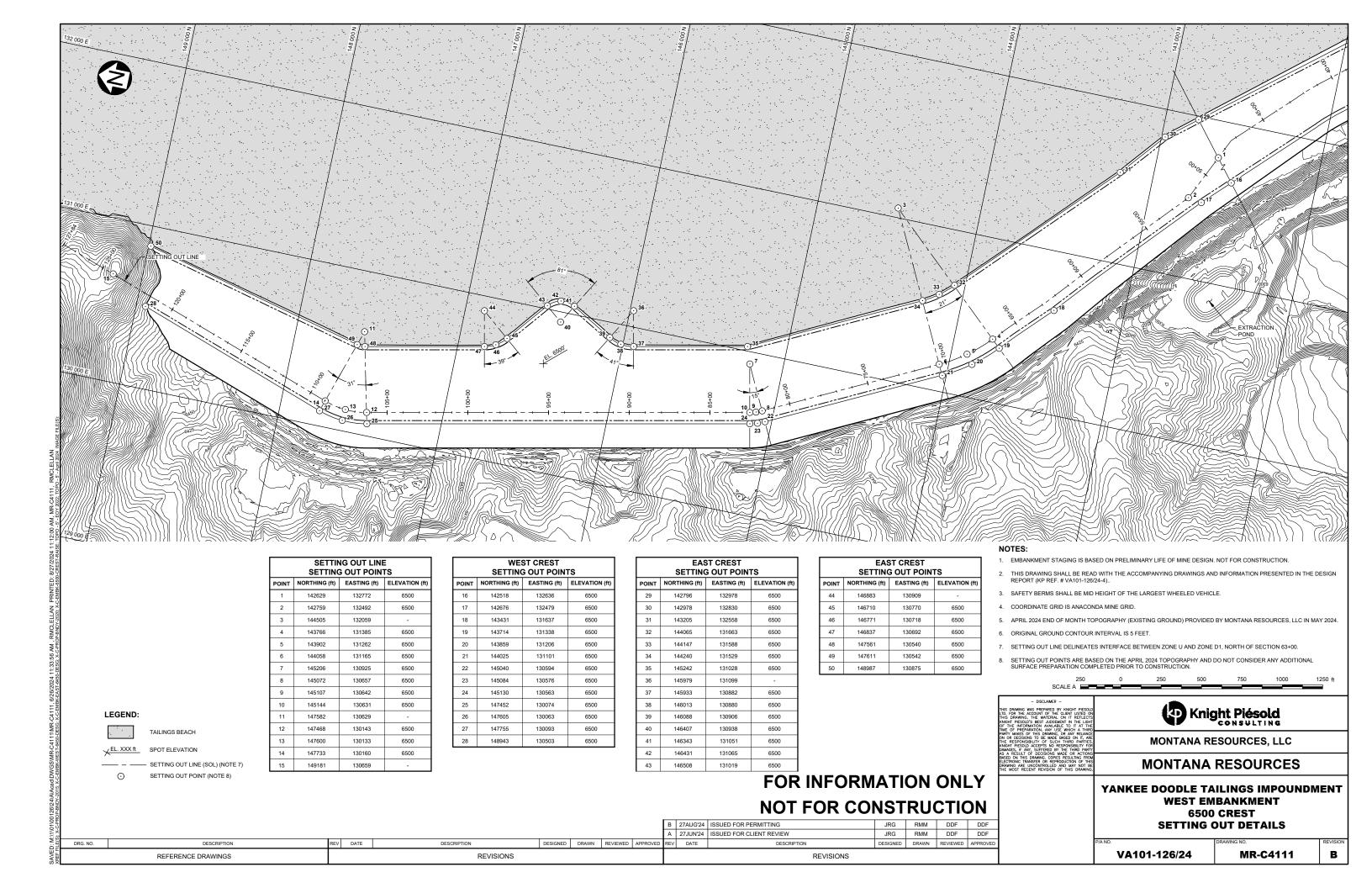
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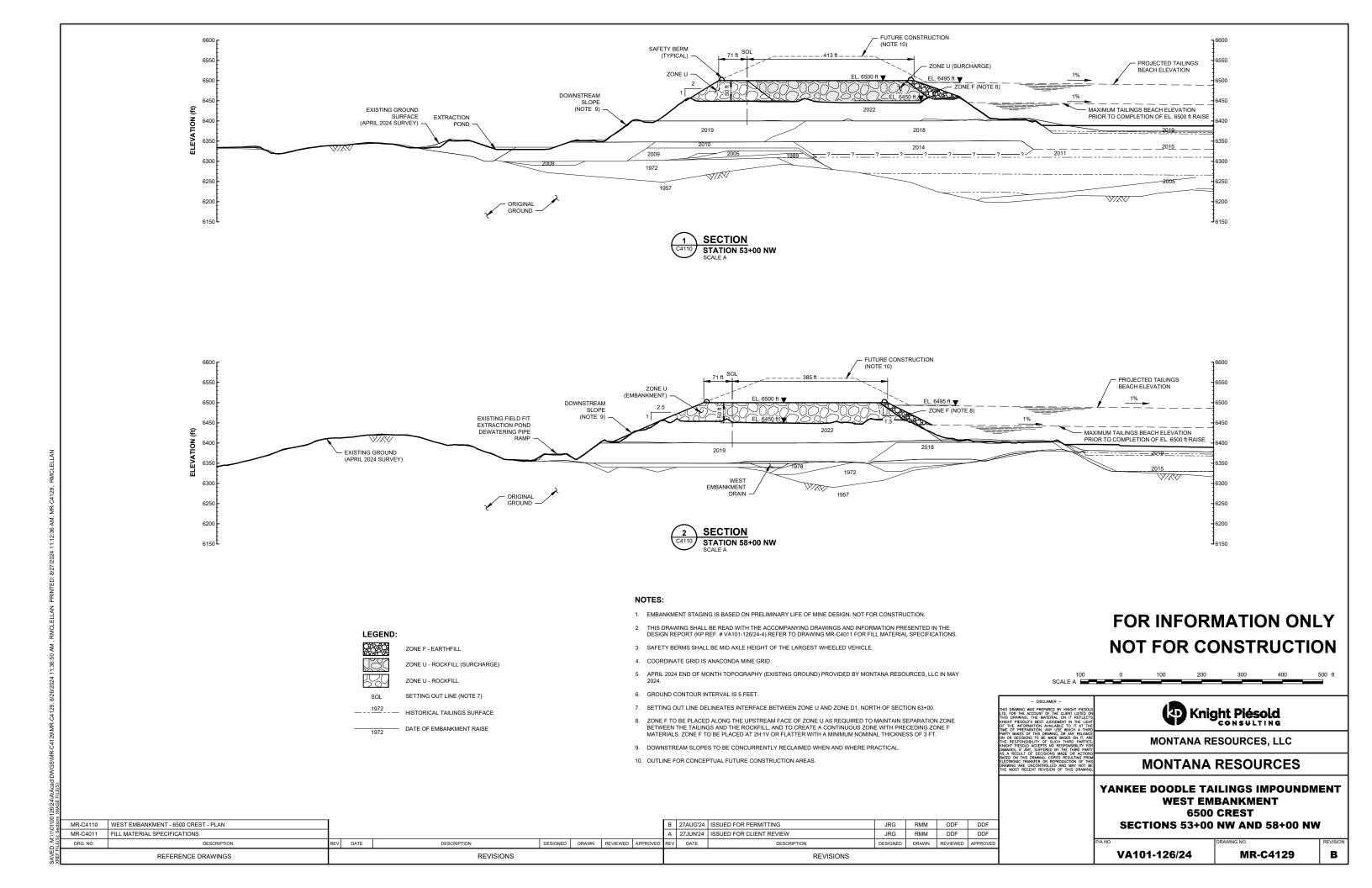
REVISIONS

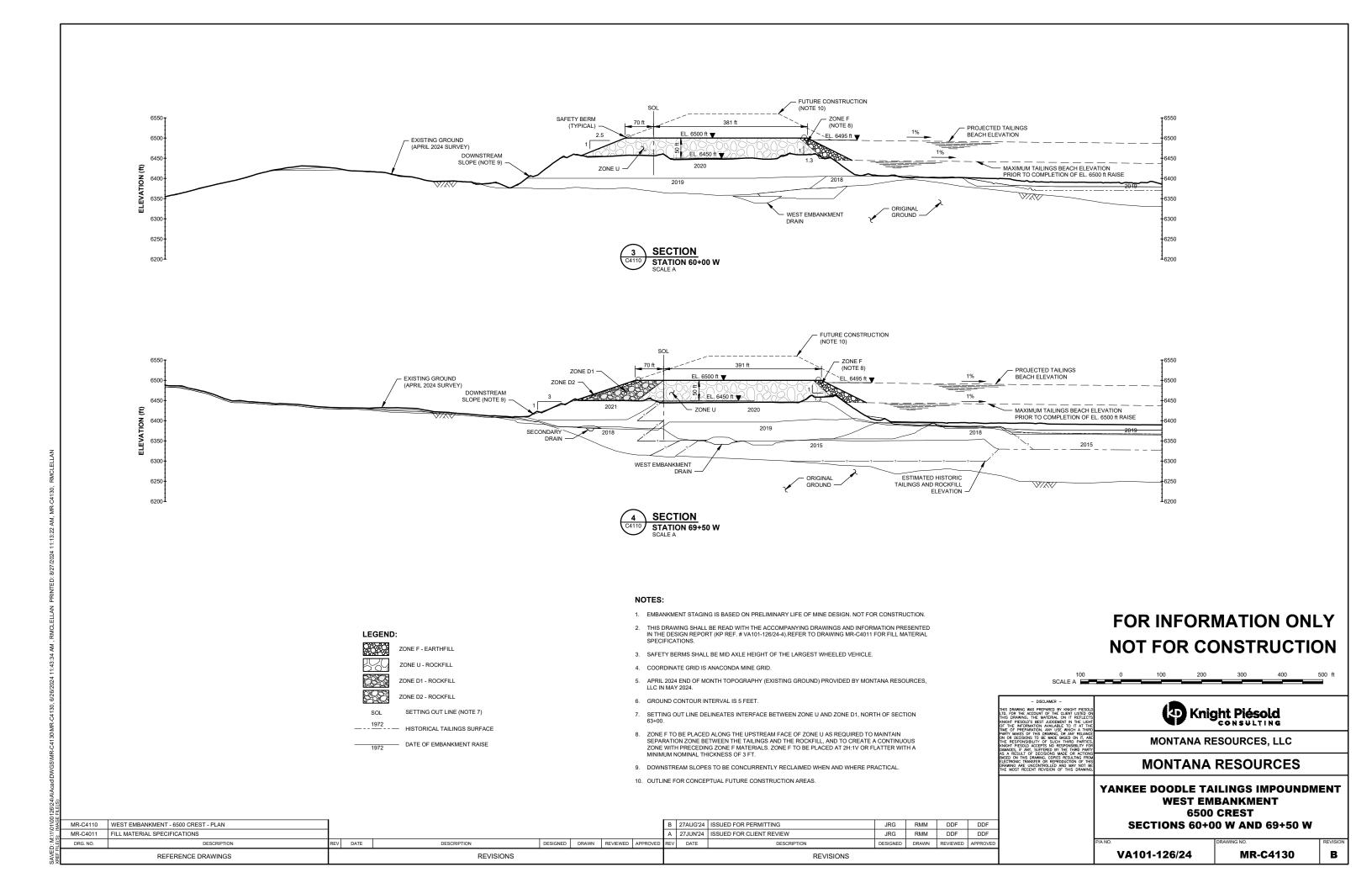
REVISIONS

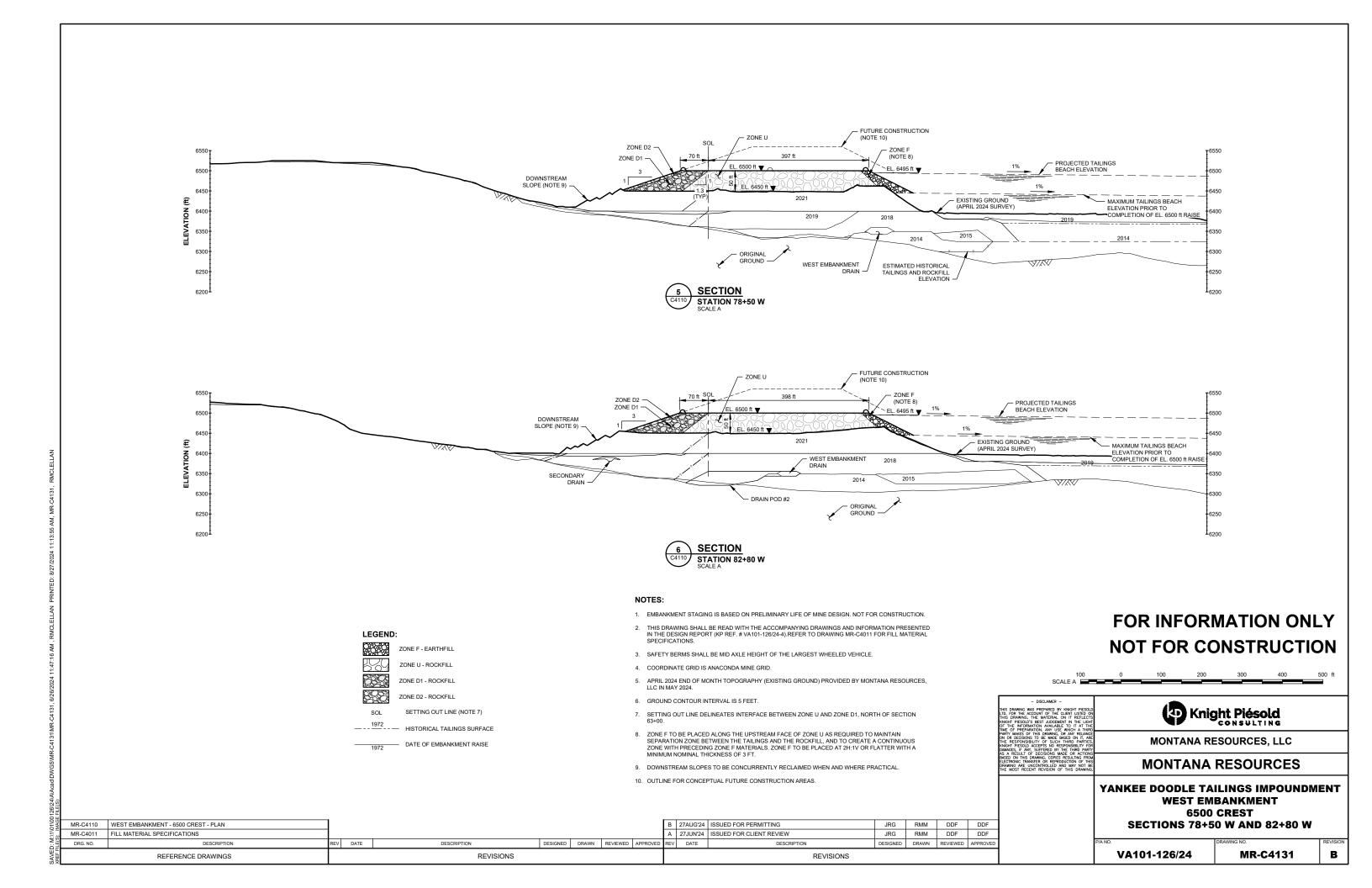


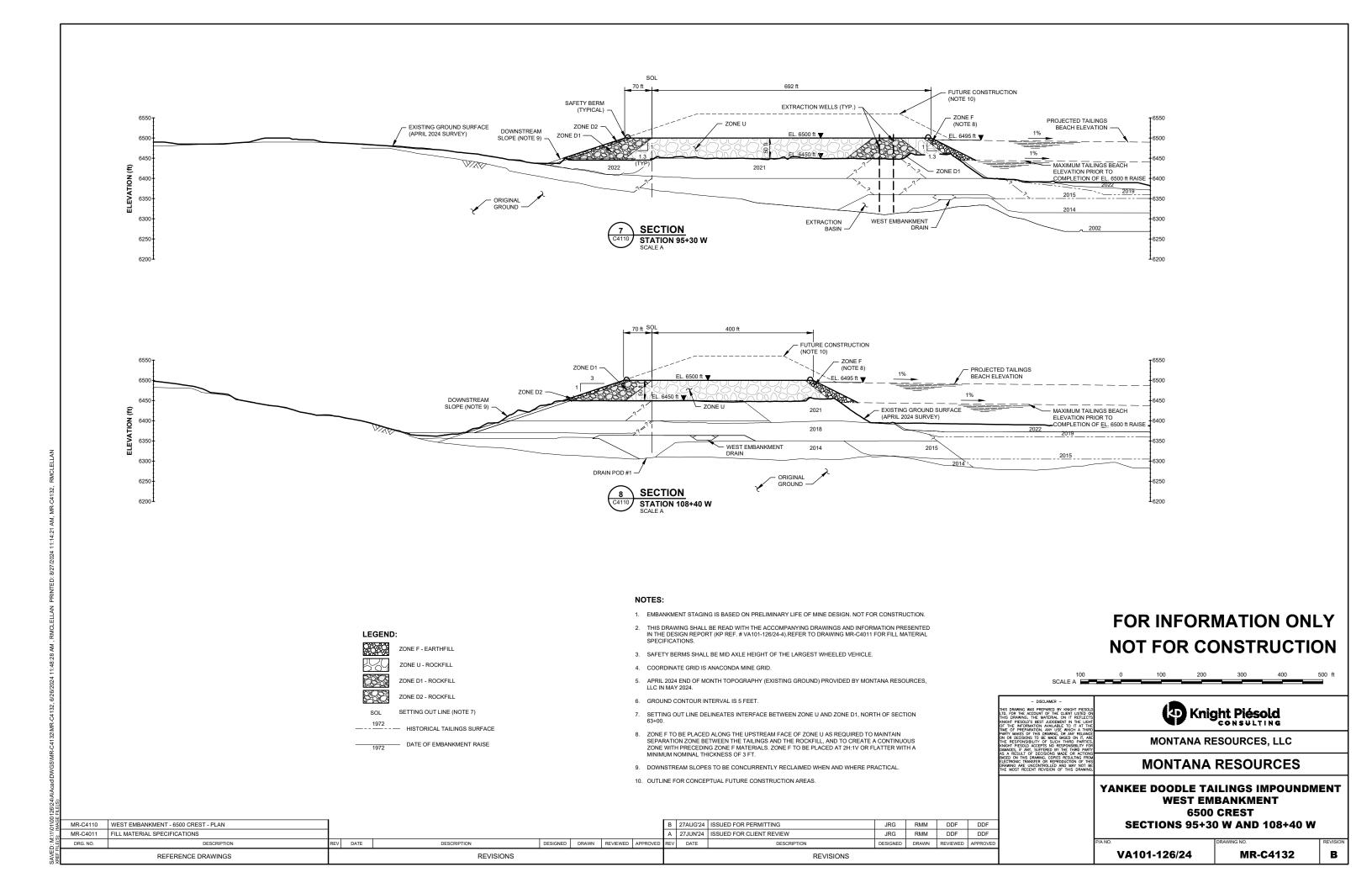


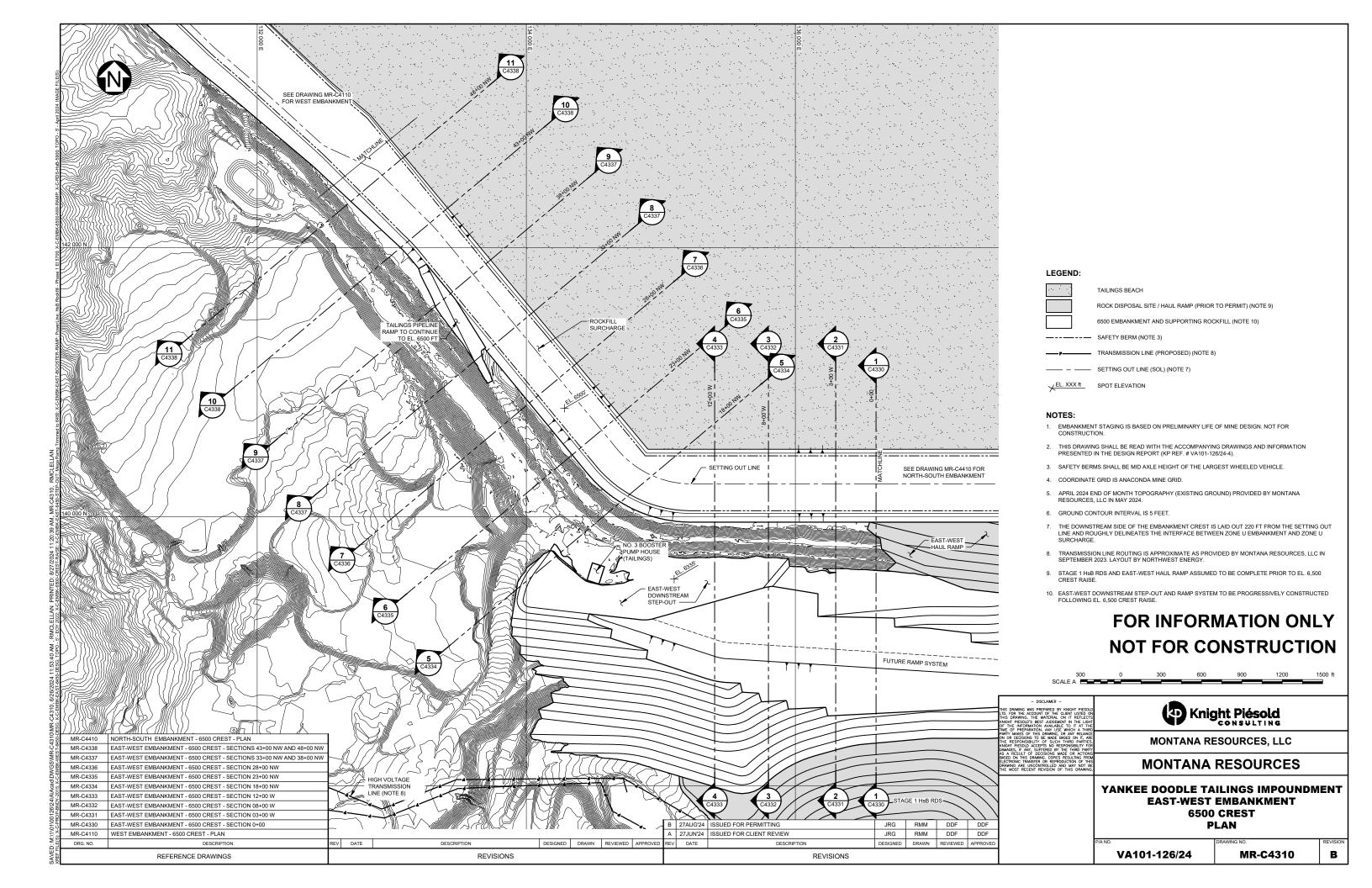


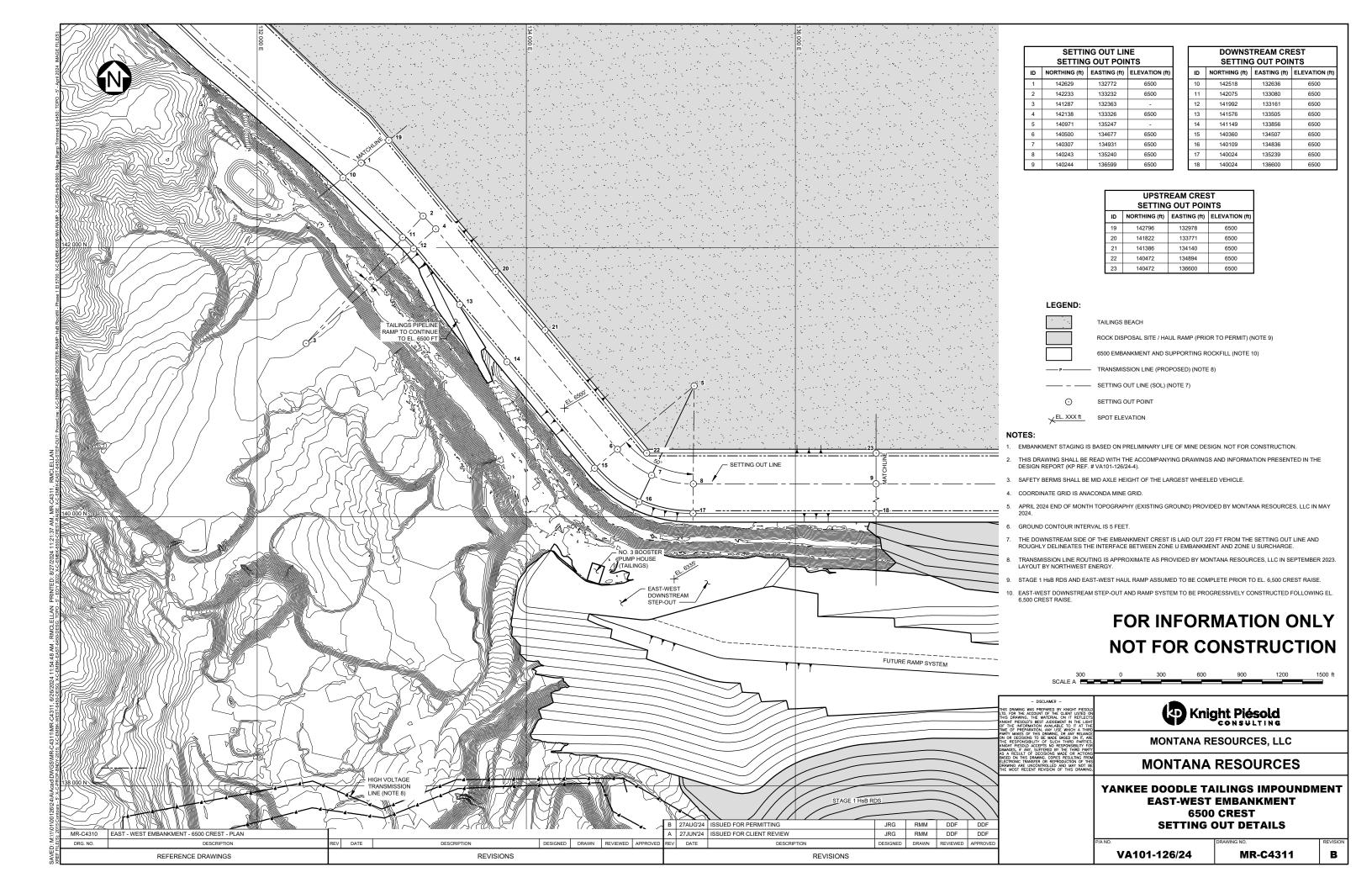


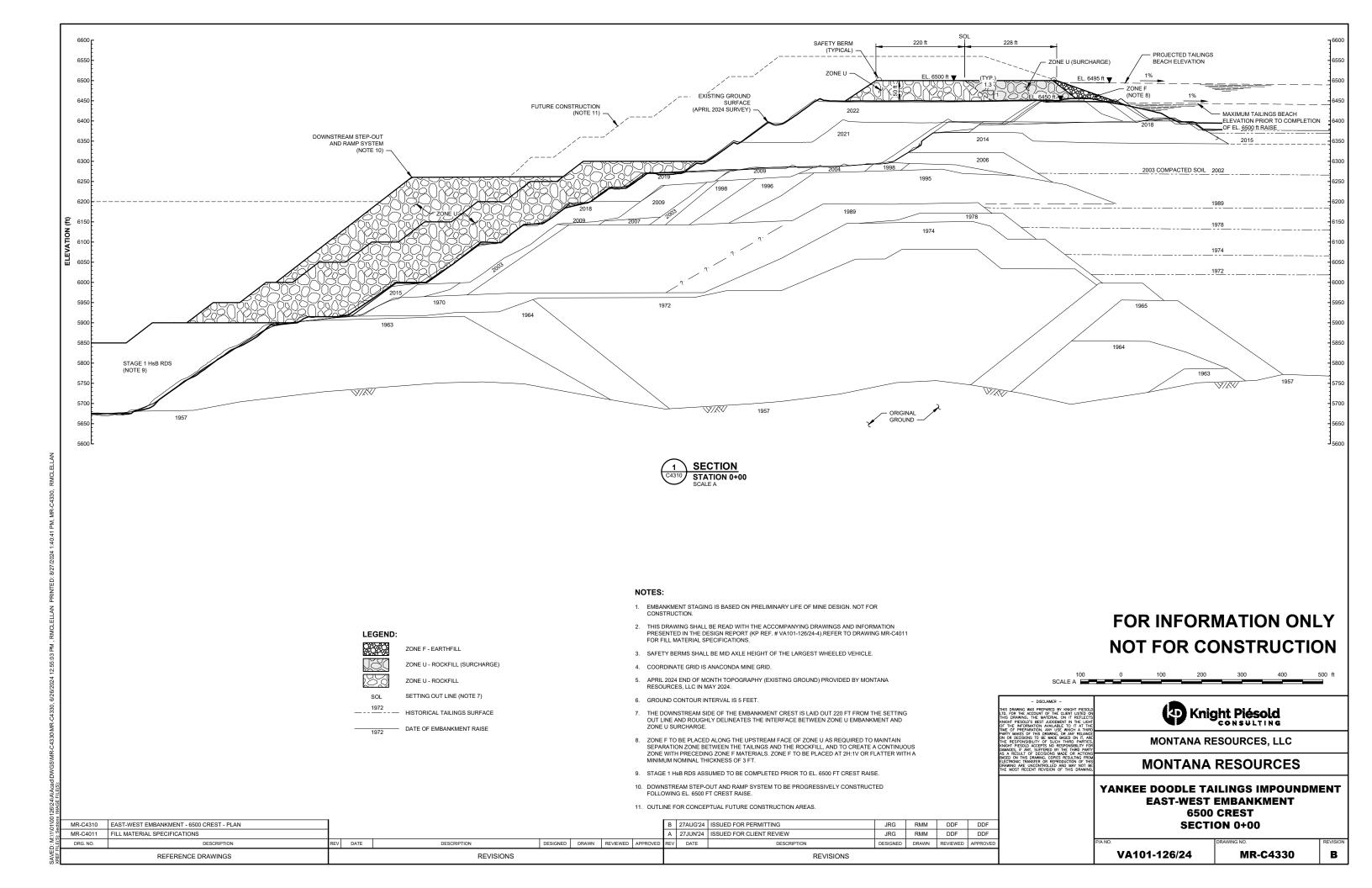


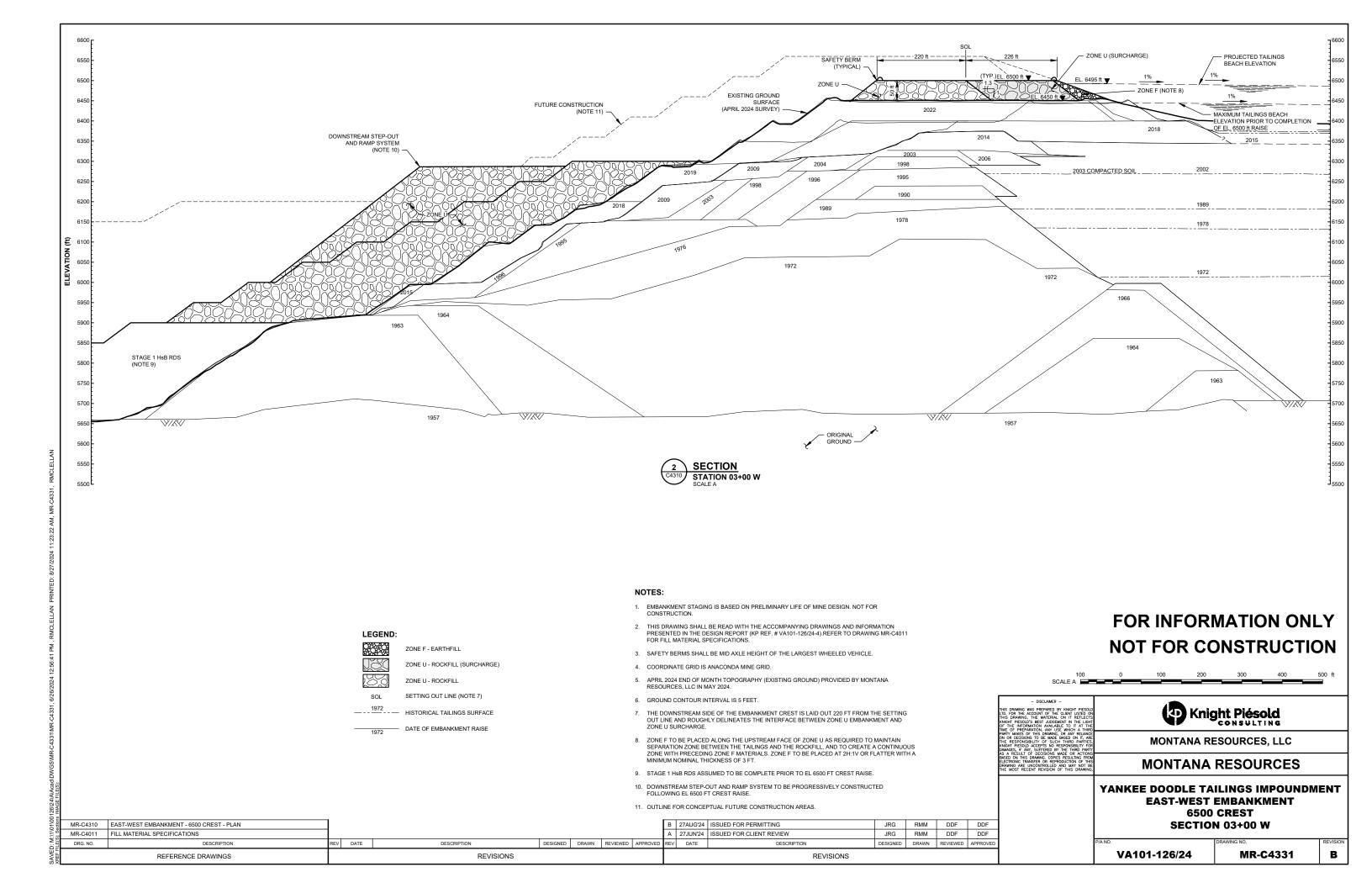


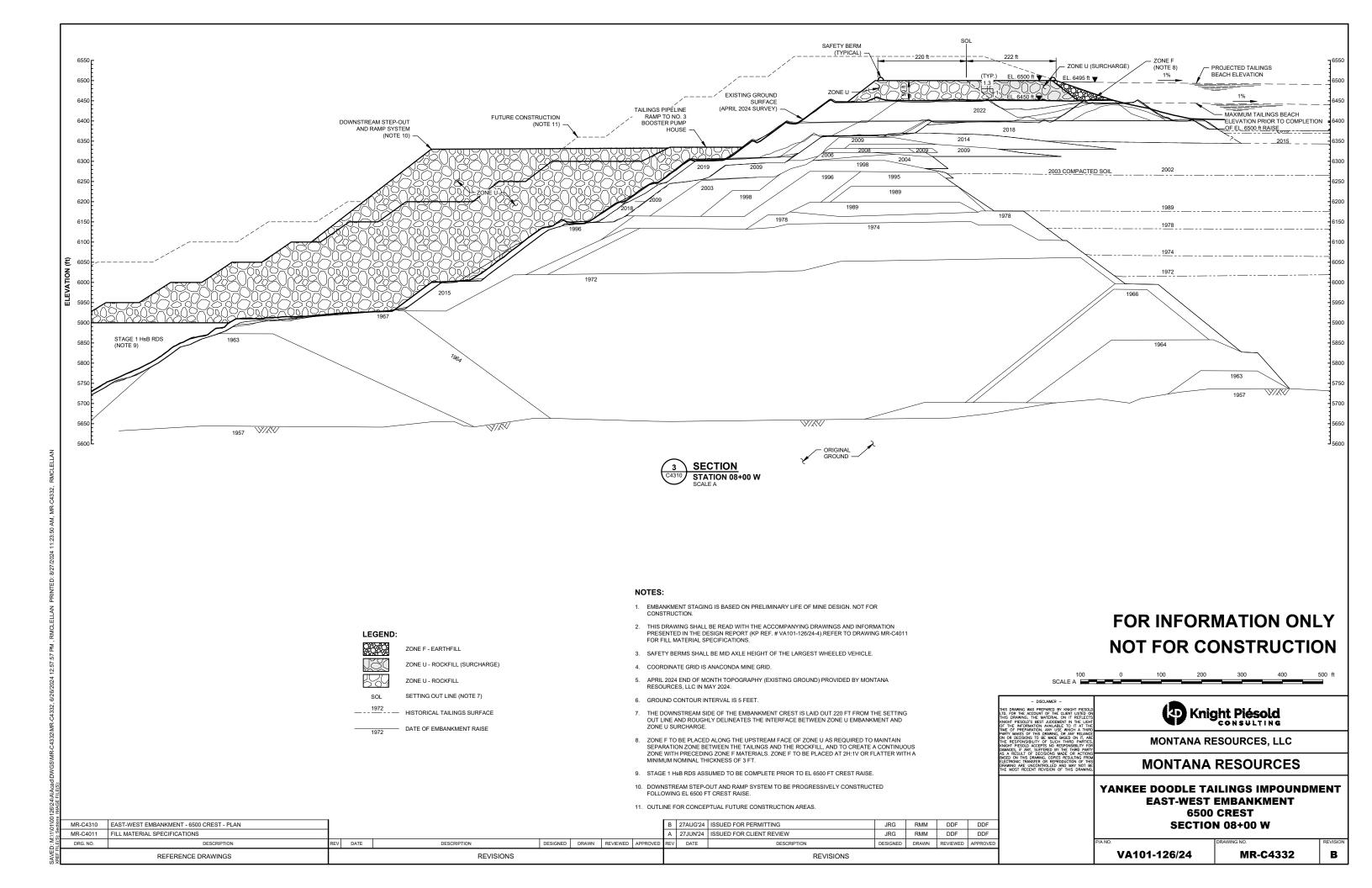


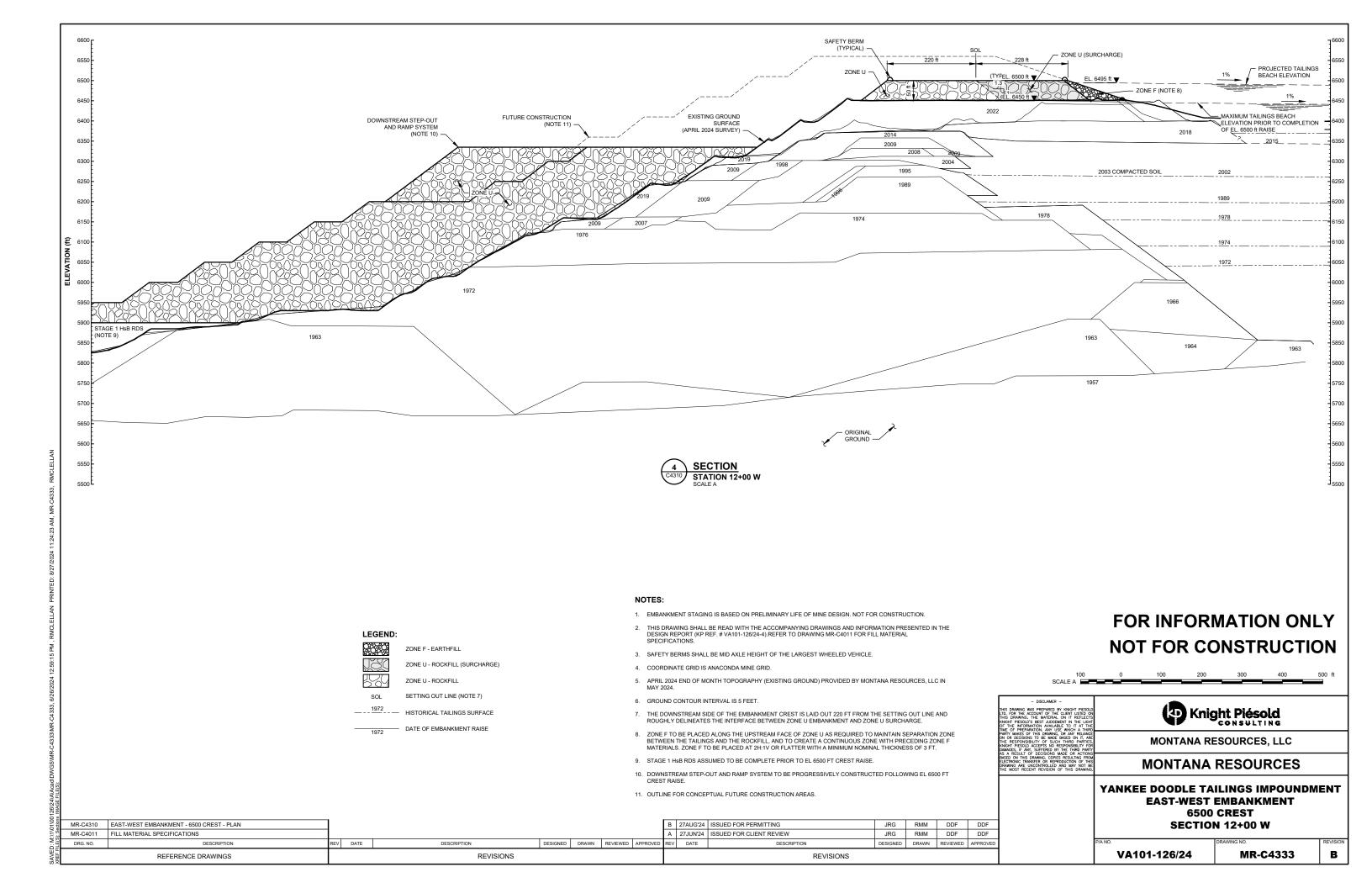


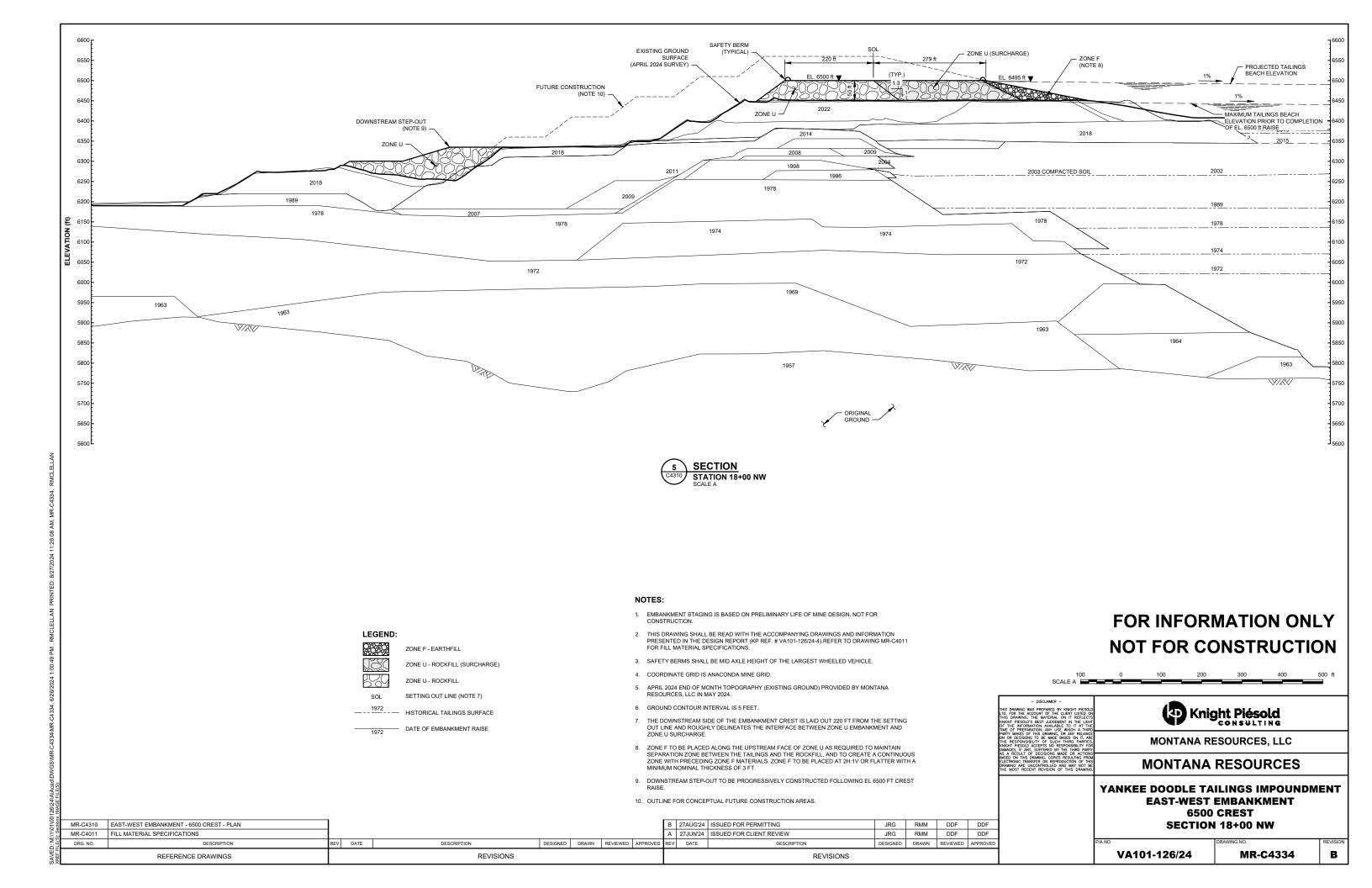


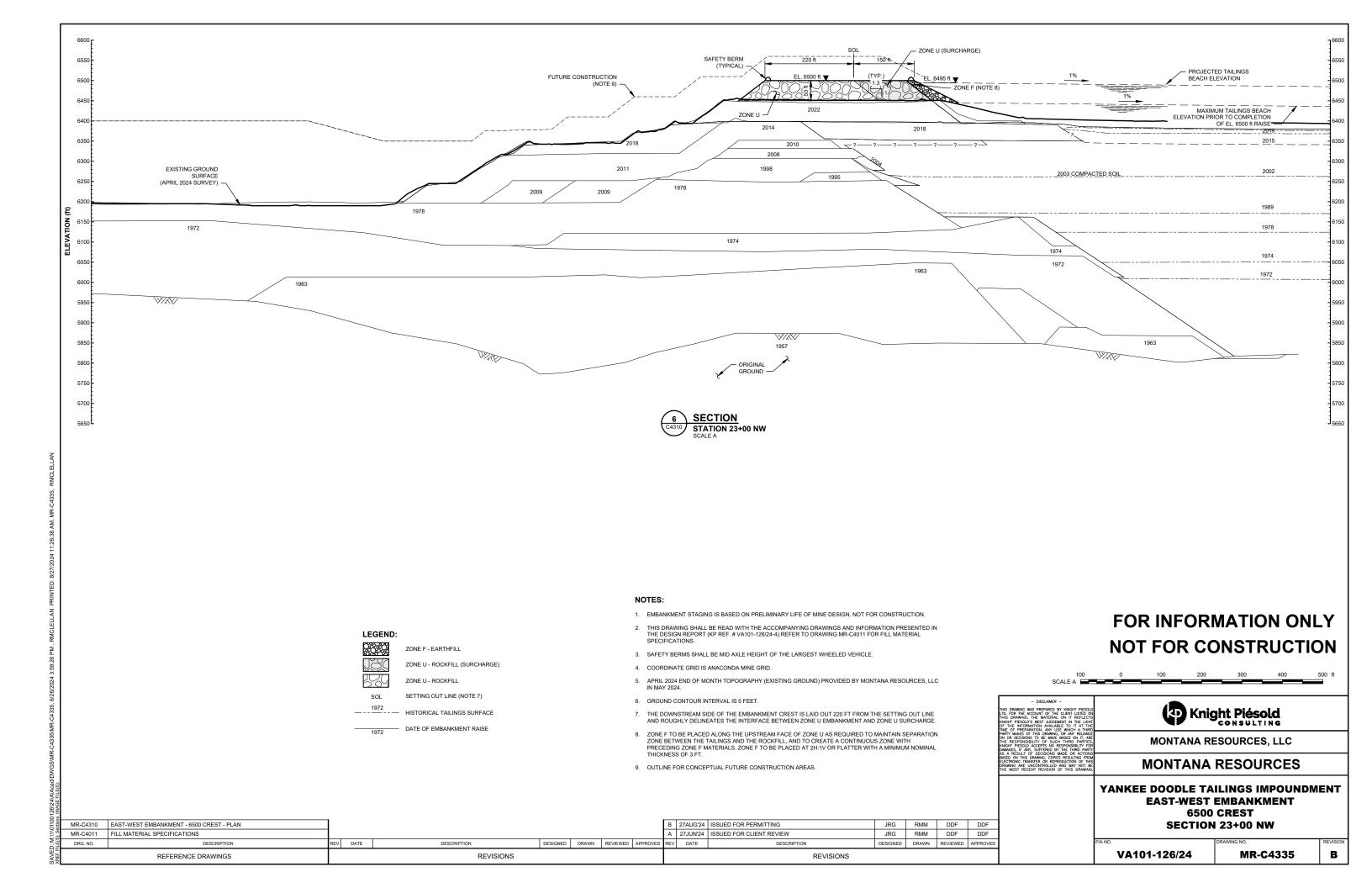


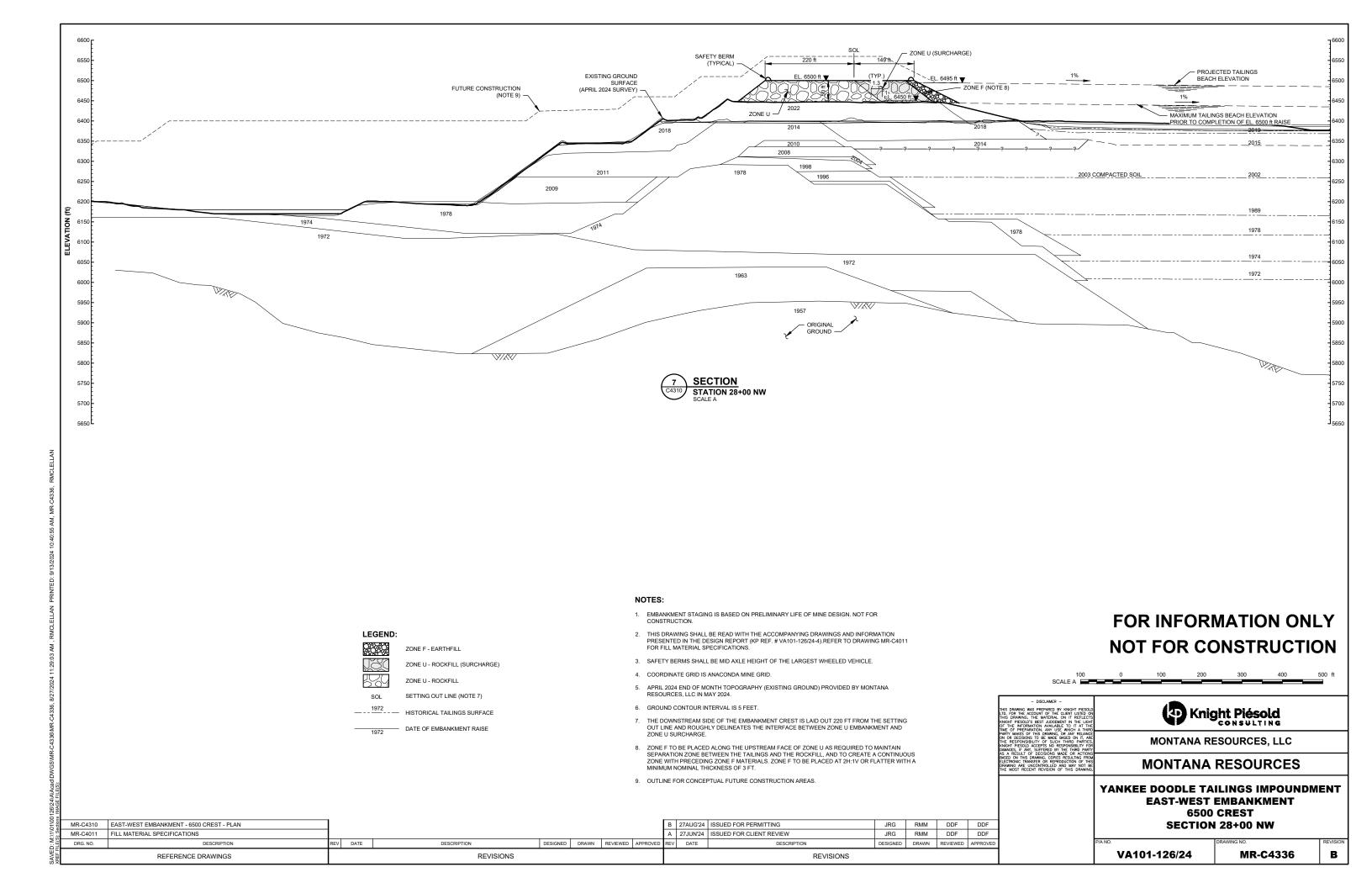


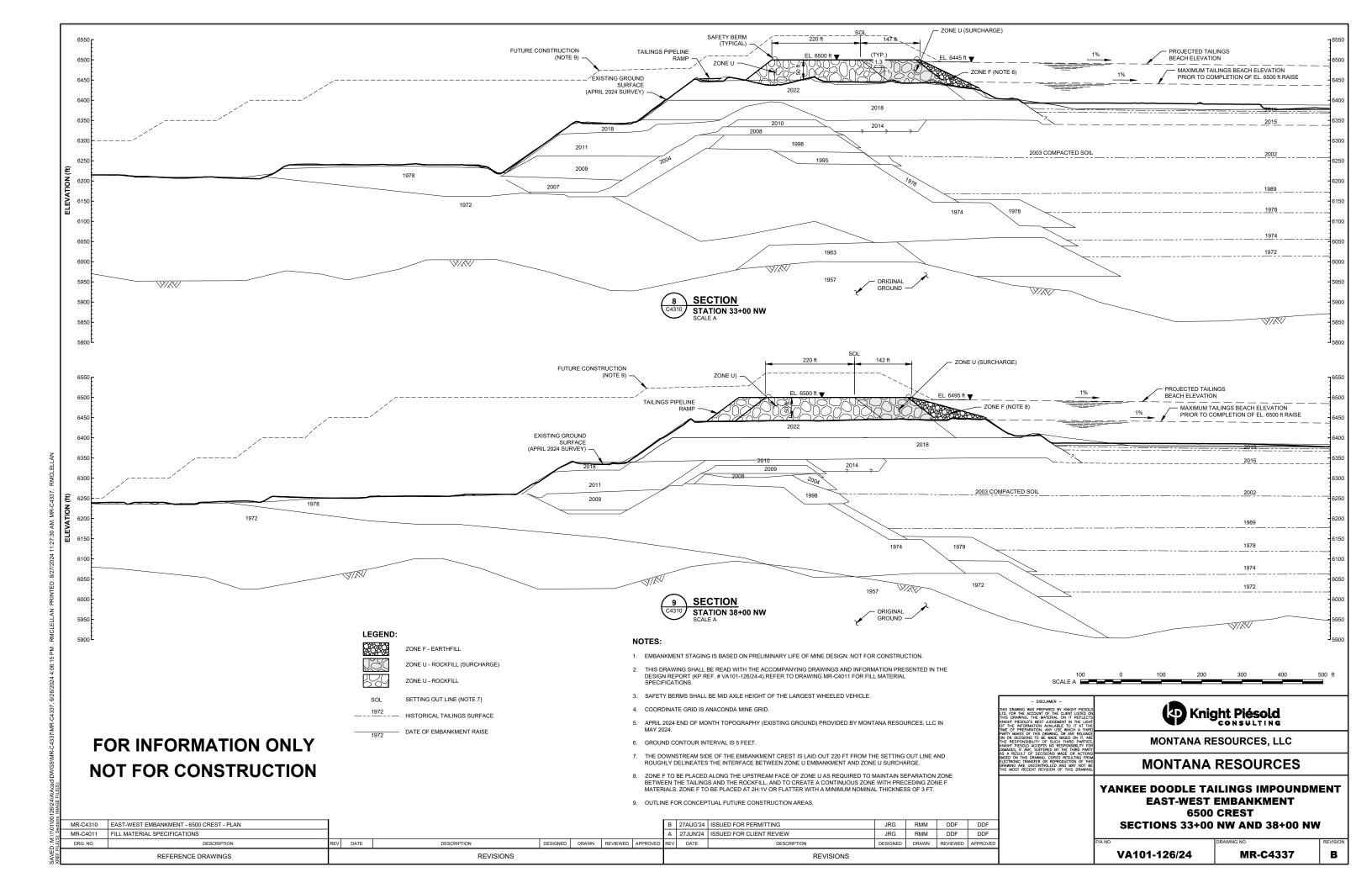


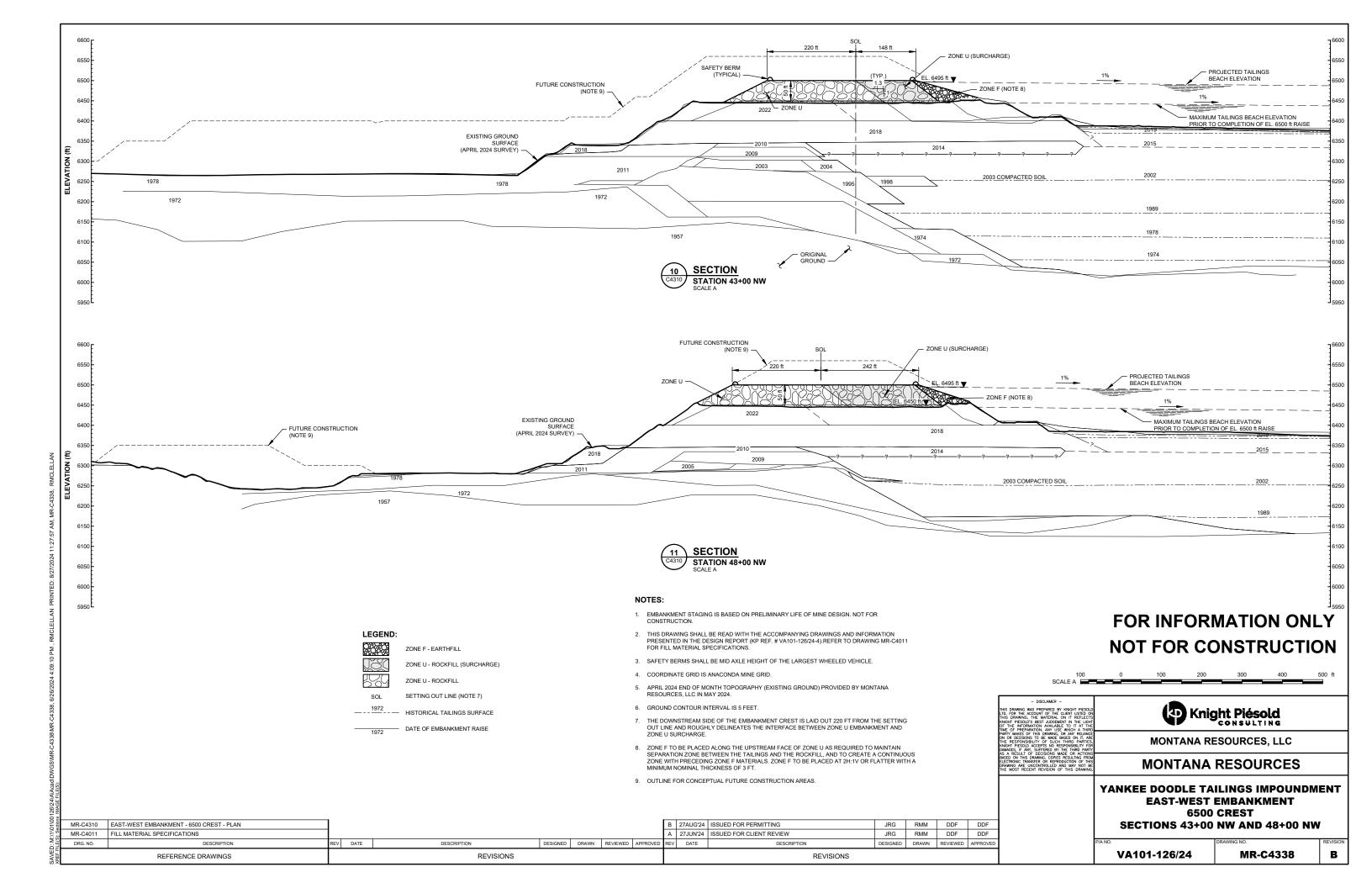


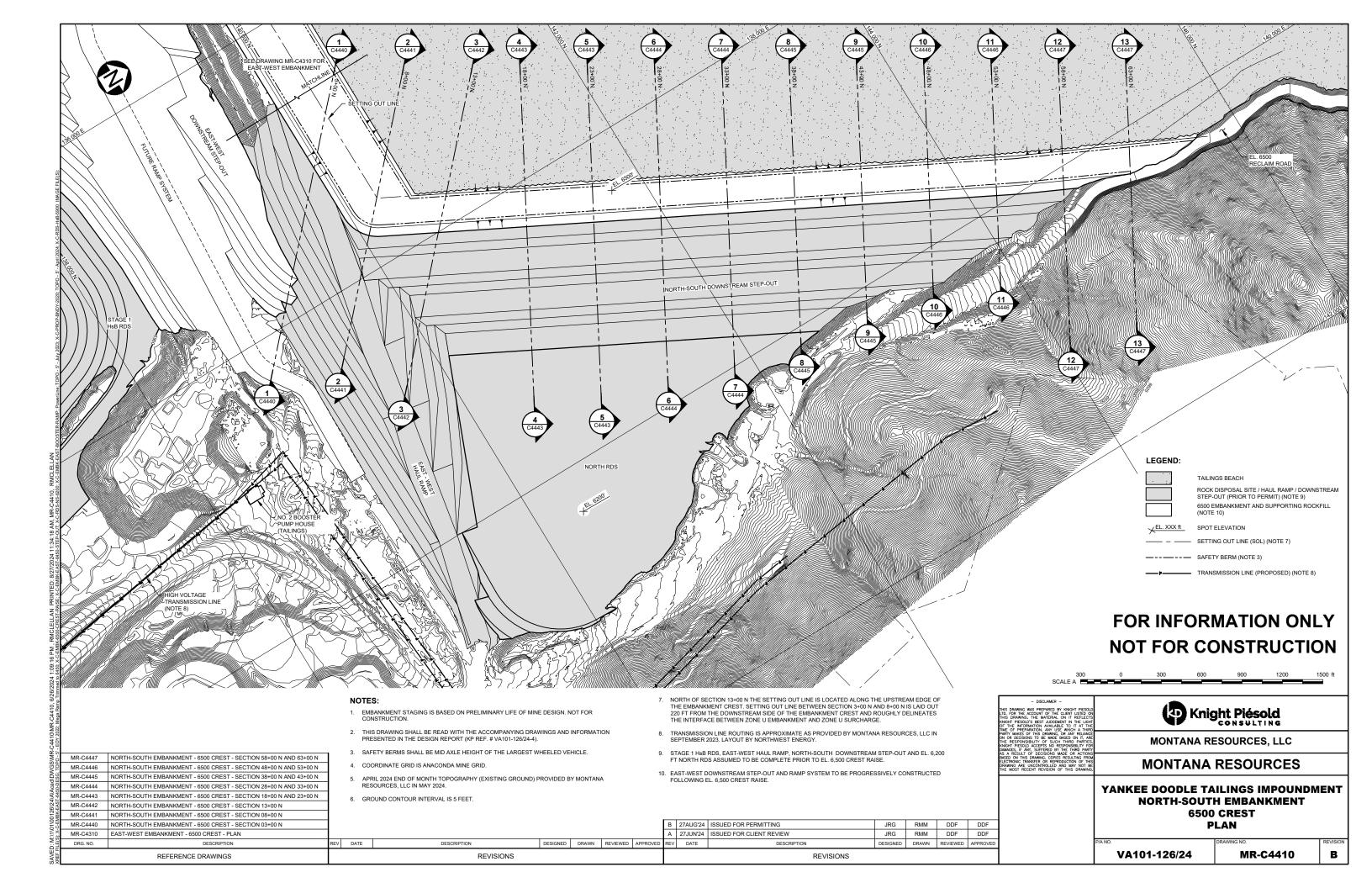


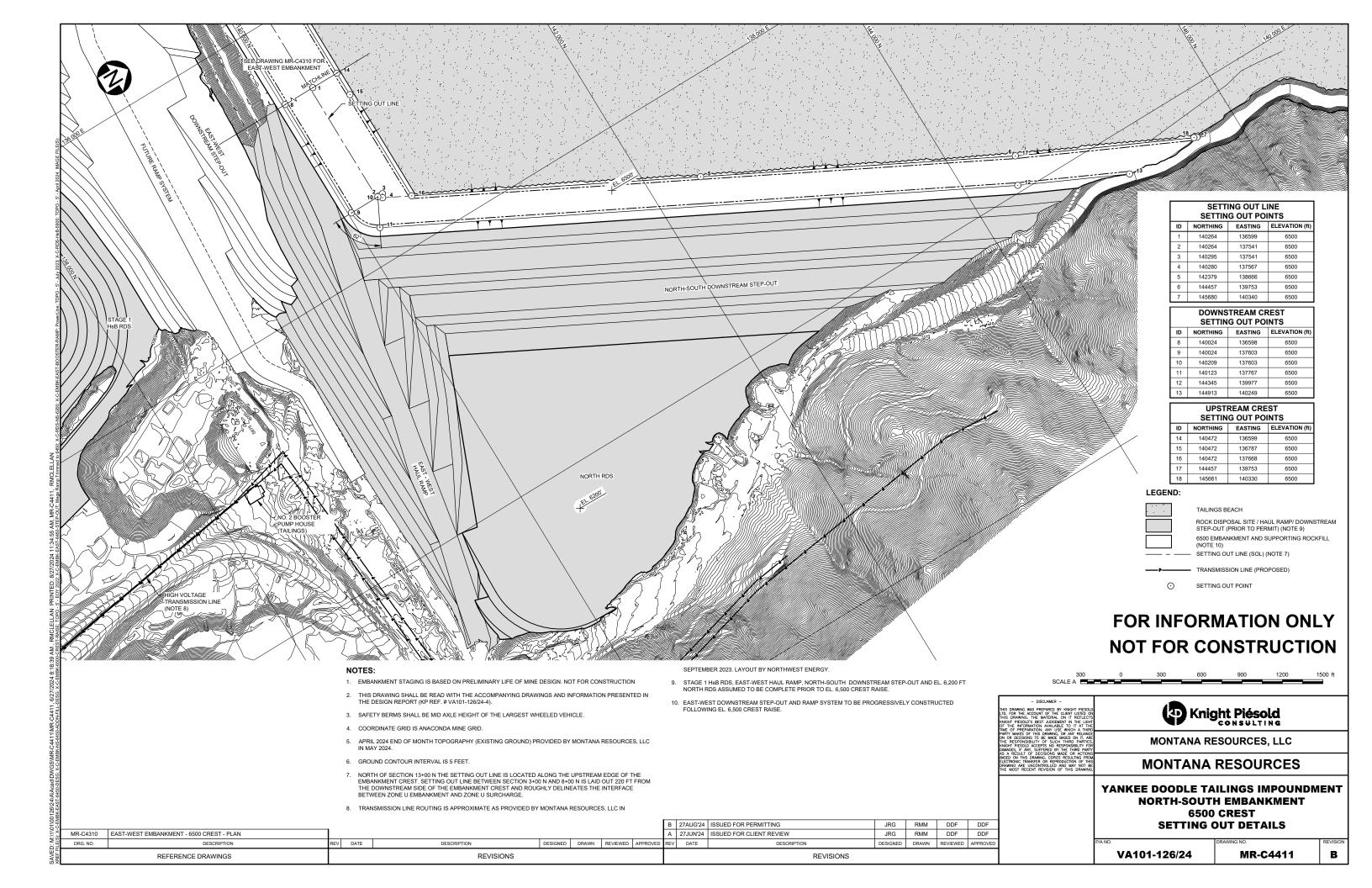


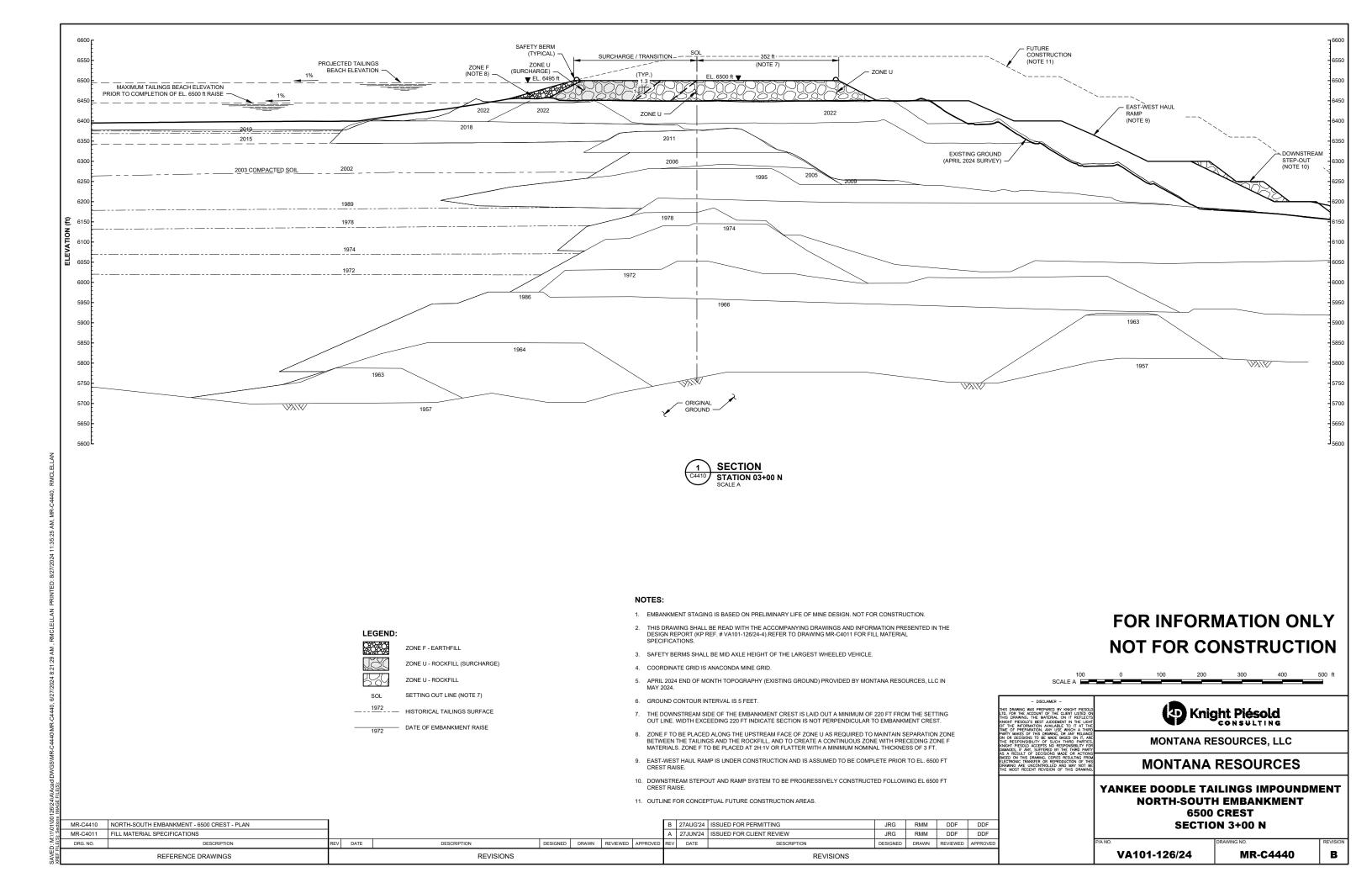


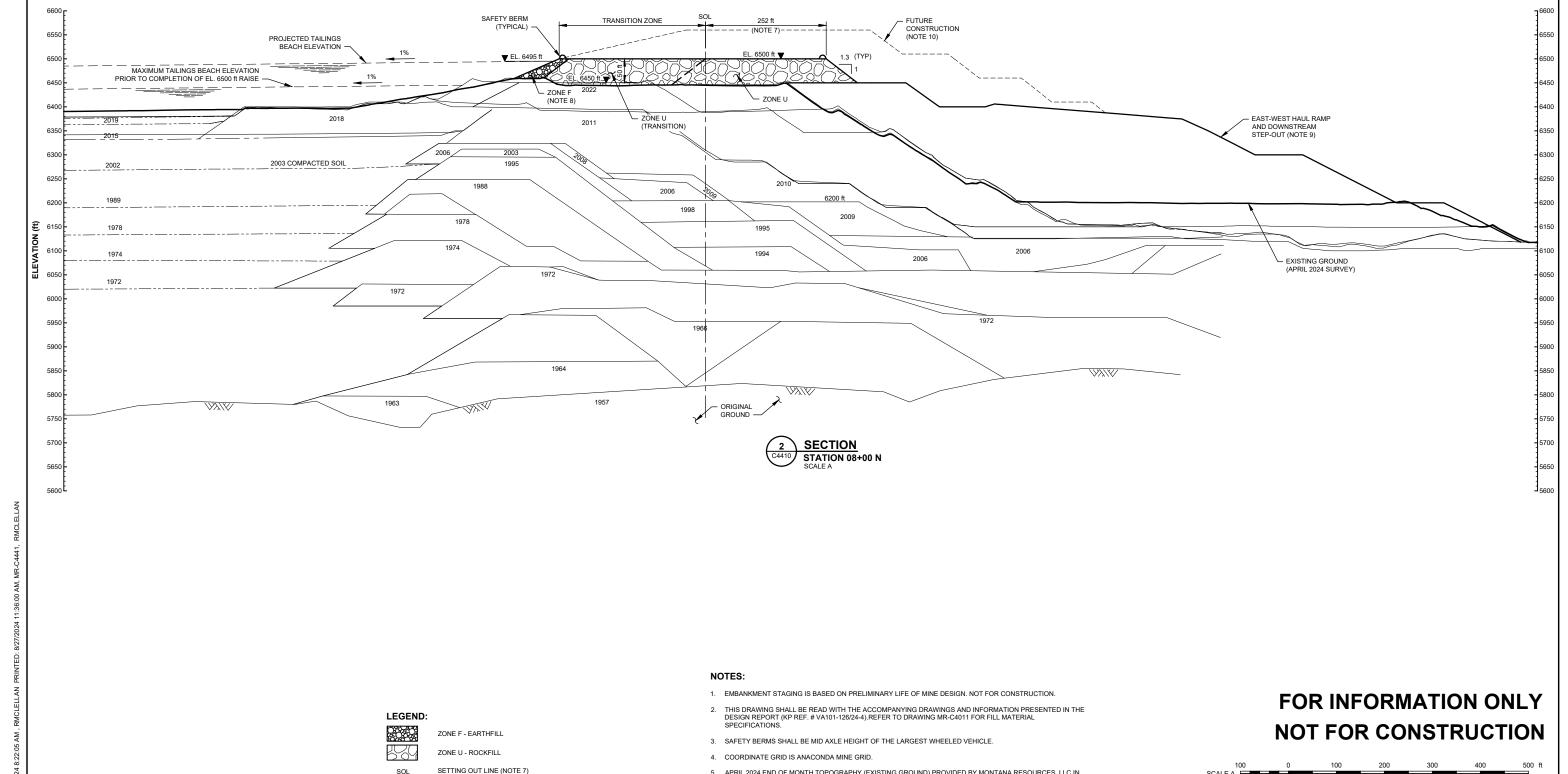












- APRIL 2024 END OF MONTH TOPOGRAPHY (EXISTING GROUND) PROVIDED BY MONTANA RESOURCES, LLC IN MAY 2024
- 6. GROUND CONTOUR INTERVAL IS 5 FEET.

1972 DATE OF EMBANKMENT RAISE

REVISIONS

REV DATE

MR-C4410 NORTH-SOUTH EMBANKMENT - 6500 CREST - PLAN

REFERENCE DRAWINGS

MR-C4011 FILL MATERIAL SPECIFICATIONS

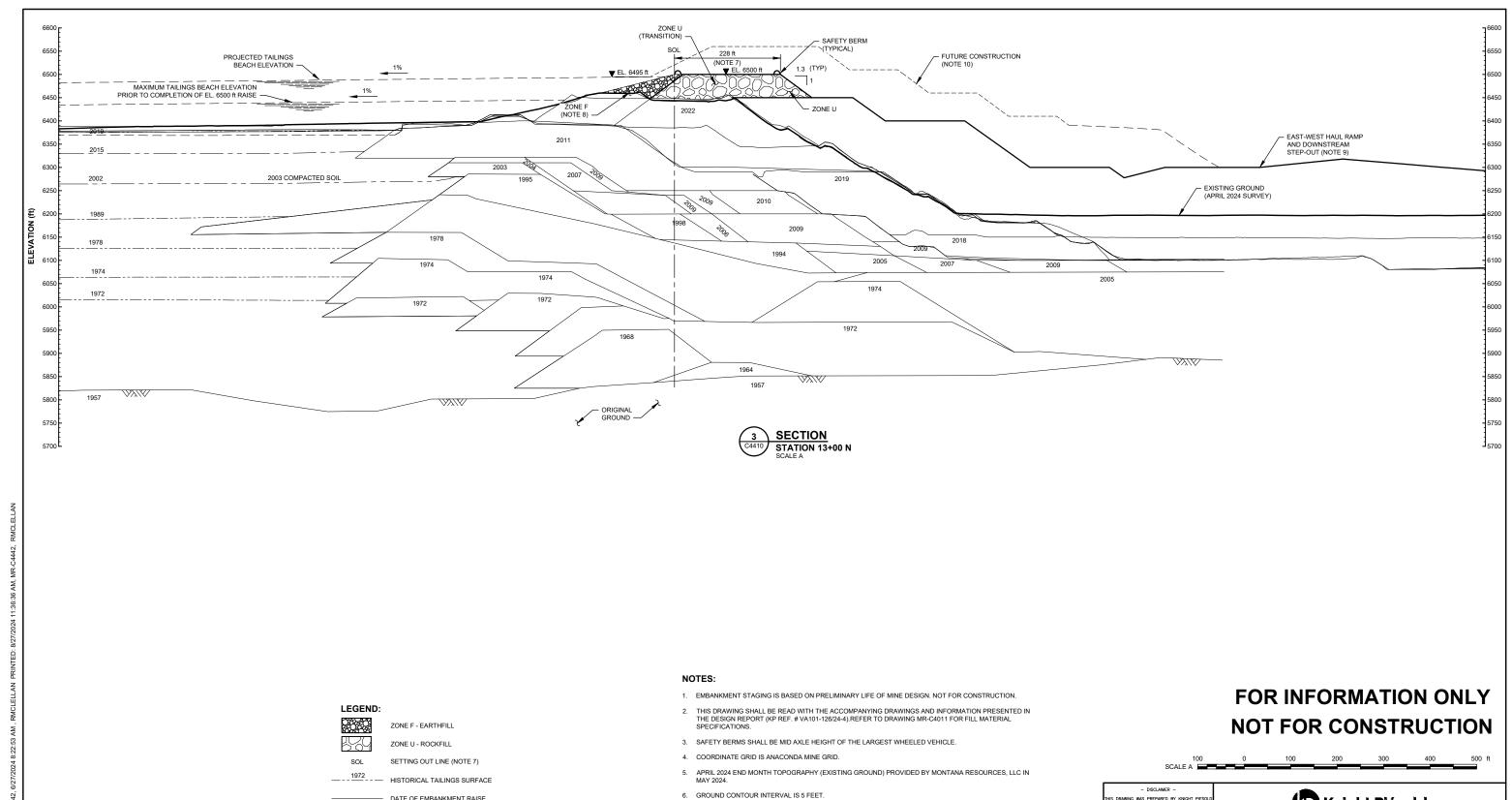
- 7. THE DOWNSTREAM SIDE OF THE EMBANKMENT CREST IS LAID OUT A MINIMUM OF 220 FT FROM THE SETTING OUT LINE. WIDTH EXCEEDING 220 FT INDICATE SECTION IS NOT PERPENDICULAR TO EMBANKMENT CREST.
- ZONE F TO BE PLACED ALONG THE UPSTREAM FACE OF ZONE U AS REQUIRED TO MAINTAIN SEPARATION ZONE BETWEEN THE TAILINGS AND THE ROCKFILL, AND TO CREATE A CONTINUOUS ZONE WITH PRECEDING ZONE F MATERIALS. ZONE F TO BE PLACED AT 2H:1V OR FLATTER WITH A MINIMUM NOMINAL THICKNESS OF 3 FT.
- EAST-WEST HAUL RAMP AND DOWNSTREAM STEP-OUT ARE UNDER CONSTRUCTION AND ARE ASSUMED TO BE COMPLETE PRIOR TO EL. 6500 FT CREST RAISE.
- 10. OUTLINE FOR CONCEPTUAL FUTURE CONSTRUCTION AREAS.

B 27AUG'24 ISSUED FOR PERMITTING JRG RMM DDF DDF A 27JUN'24 ISSUED FOR CLIENT REVIEW JRG RMM DDF DDF DESIGNED DRAWN REVIEWED APPROVED REV DATE DESCRIPTION DESIGNED DRAWN REVIEWED APPROVED REVISIONS



Knight Piésold

MAKES OF THIS DRAWING, OR ANY RELIANCE DECISIONS TO BE MADE BASED ON IT, ARE ESPONSIBILITY OF SUCH THIRD PARTIES. PIESOLD ACCEPTS NO RESPONSIBILITY FOR IS, IF ANY, SUFFERED BY THE THIRD PARTY FESULIT OF DECISIONS MADE OR ACTIONS	MONTANA RESOURCES, LLC			
RESULT OF DECISIONS MADE OR ACTIONS ON THIS DRAWING. COPIES RESULTING FROM ONIC TRANSFER OR REPRODUCTION OF THIS IG ARE UNCONTROLLED AND MAY NOT BE OST RECENT REVISION OF THIS DRAWING.	MONTANA	RESOURCES		
	YANKEE DOODLE TAILINGS IMPOUNDMEN NORTH-SOUTH EMBANKMENT 6500 CREST SECTION 8+00 N			
	P/A NO.	DRAWING NO.	REVISION	
	VA101-126/24	MR-C4441	В	



10. OUTLINE FOR CONCEPTUAL FUTURE CONSTRUCTION AREAS.

B 27AUG'24 ISSUED FOR PERMITTING

DESIGNED DRAWN REVIEWED APPROVED

REVISIONS

A 27JUN'24 ISSUED FOR CLIENT REVIEW

DATE OF EMBANKMENT RAISE

1972

REV DATE

MR-C4410 NORTH-SOUTH EMBANKMENT - 6500 CREST - PLAN

REFERENCE DRAWINGS

MR-C4011 FILL MATERIAL SPECIFICATIONS

7. THE DOWNSTREAM SIDE OF THE EMBANKMENT CREST IS LAID OUT A MINIMUM OF 220 FT FROM THE SETTING OUT LINE. WIDTH EXCEEDING 220 FT INDICATE SECTION IS NOT PERPENDICULAR TO EMBANKMENT CREST. 8. ZONE F TO BE PLACED ALONG THE UPSTREAM FACE OF ZONE U AS REQUIRED TO MAINTAIN SEPARATION ZONE BETWEEN THE TAILINGS AND THE ROCKFILL, AND TO CREATE A CONTINUOUS ZONE WITH PRECEDING ZONE F MATERIALS. ZONE F TO BE PLACED AT 2H:1V OR FLATTER WITH A MINIMUM NOMINAL THICKNESS OF 9. EAST-WEST HAUL RAMP AND DOWNSTREAM STEP-OUT ARE UNDER CONSTRUCTION AND ARE ASSUMED TO BE COMPLETE PRIOR TO EL. 6500 FT CREST RAISE. JRG RMM DDF DDF JRG RMM DDF DDF

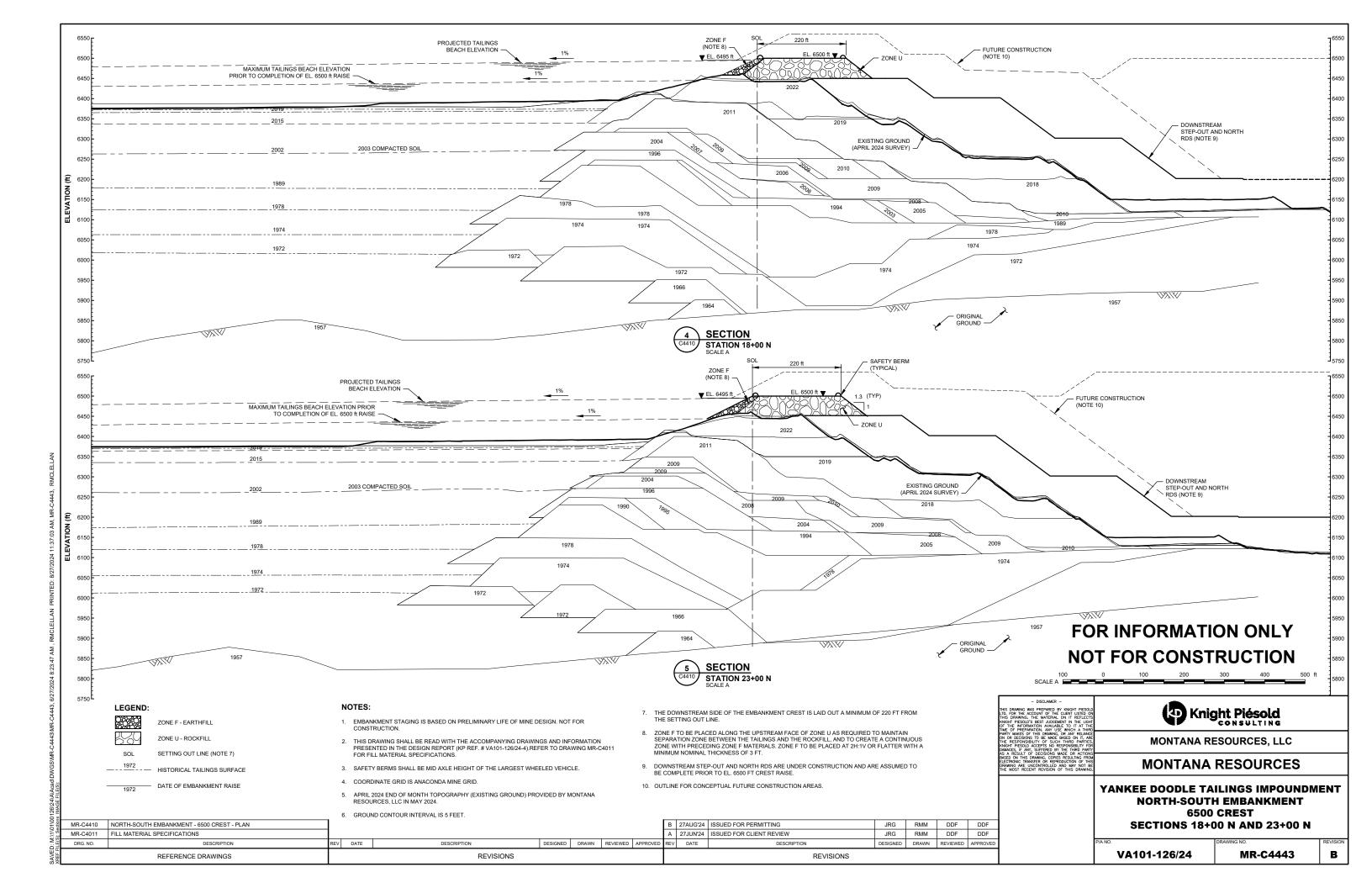
DESIGNED DRAWN REVIEWED APPROVE

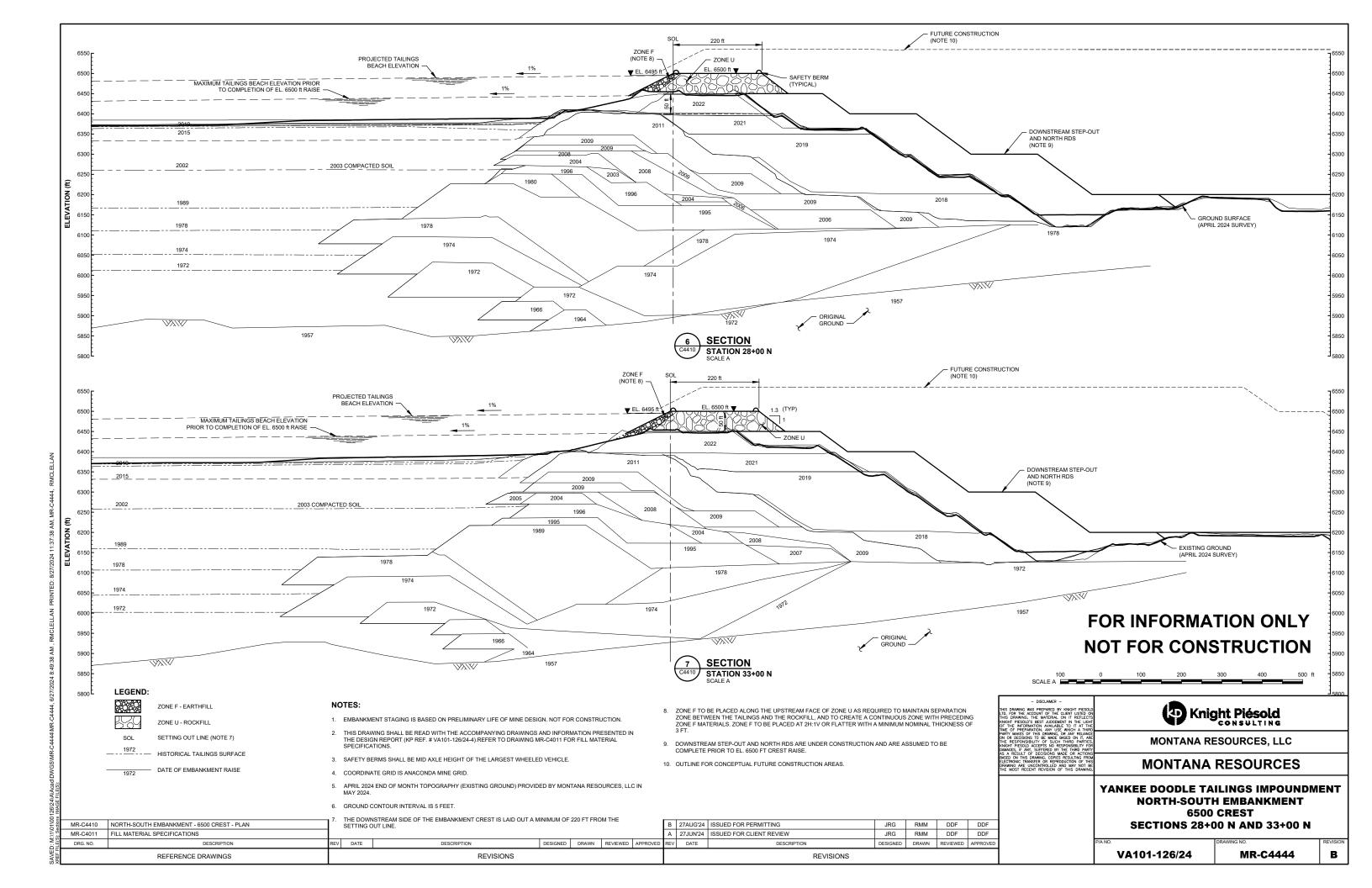
REVISIONS

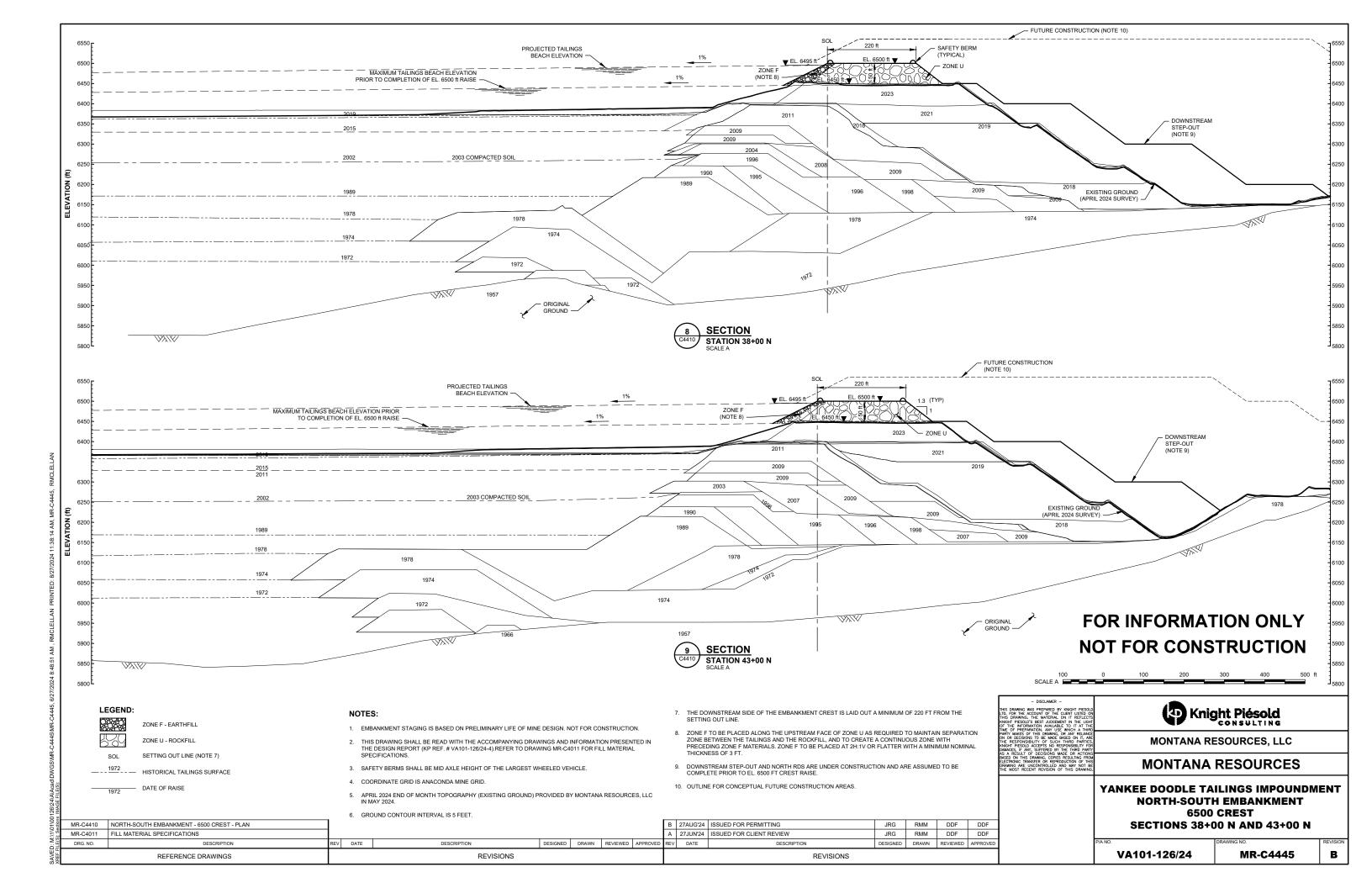
Knight Piésold

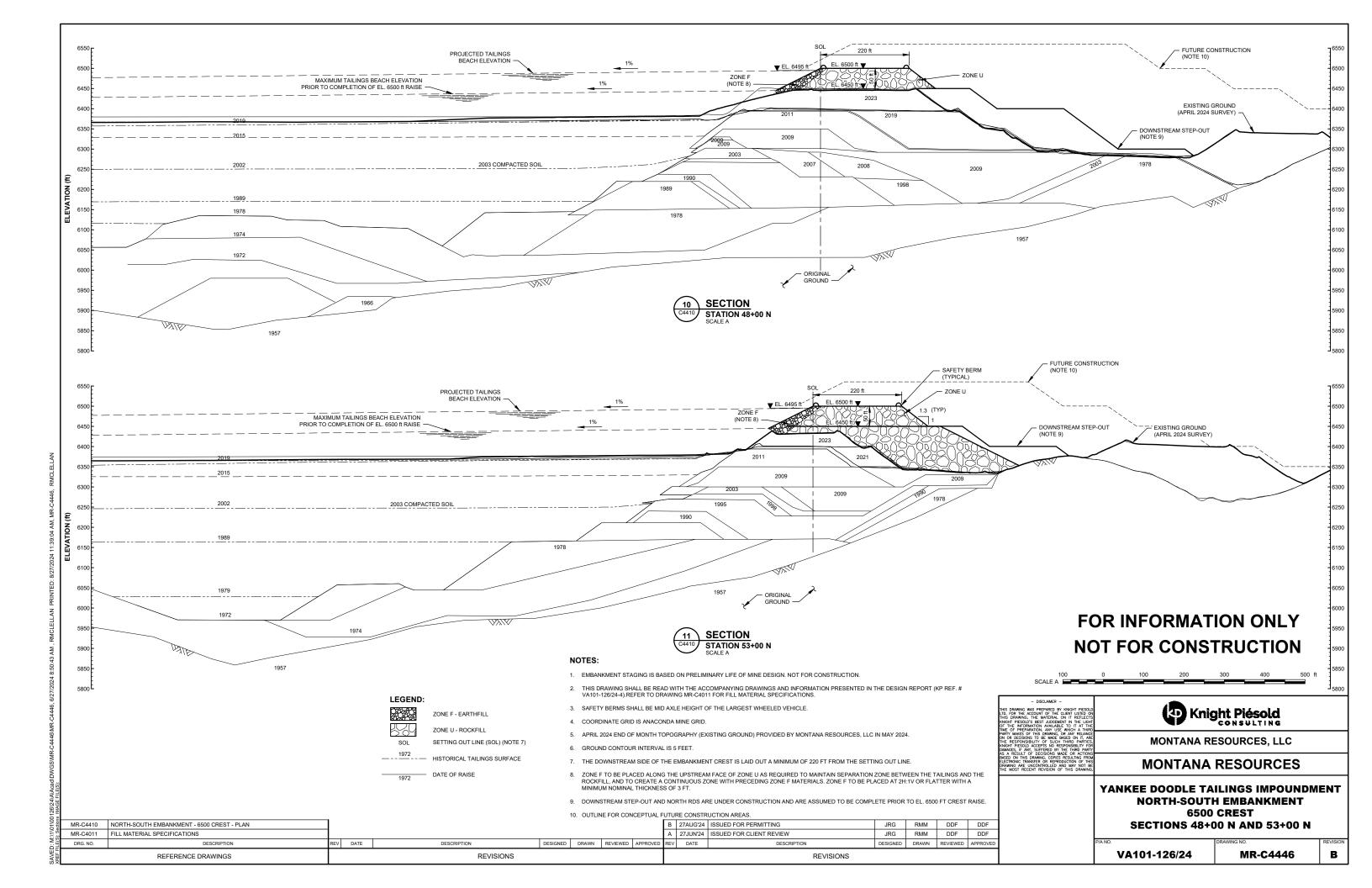
MONTANA RESOURCES, LLC MONTANA RESOURCES YANKEE DOODLE TAILINGS IMPOUNDMENT **NORTH-SOUTH EMBANKMENT 6500 CREST** SECTION 13+00 N

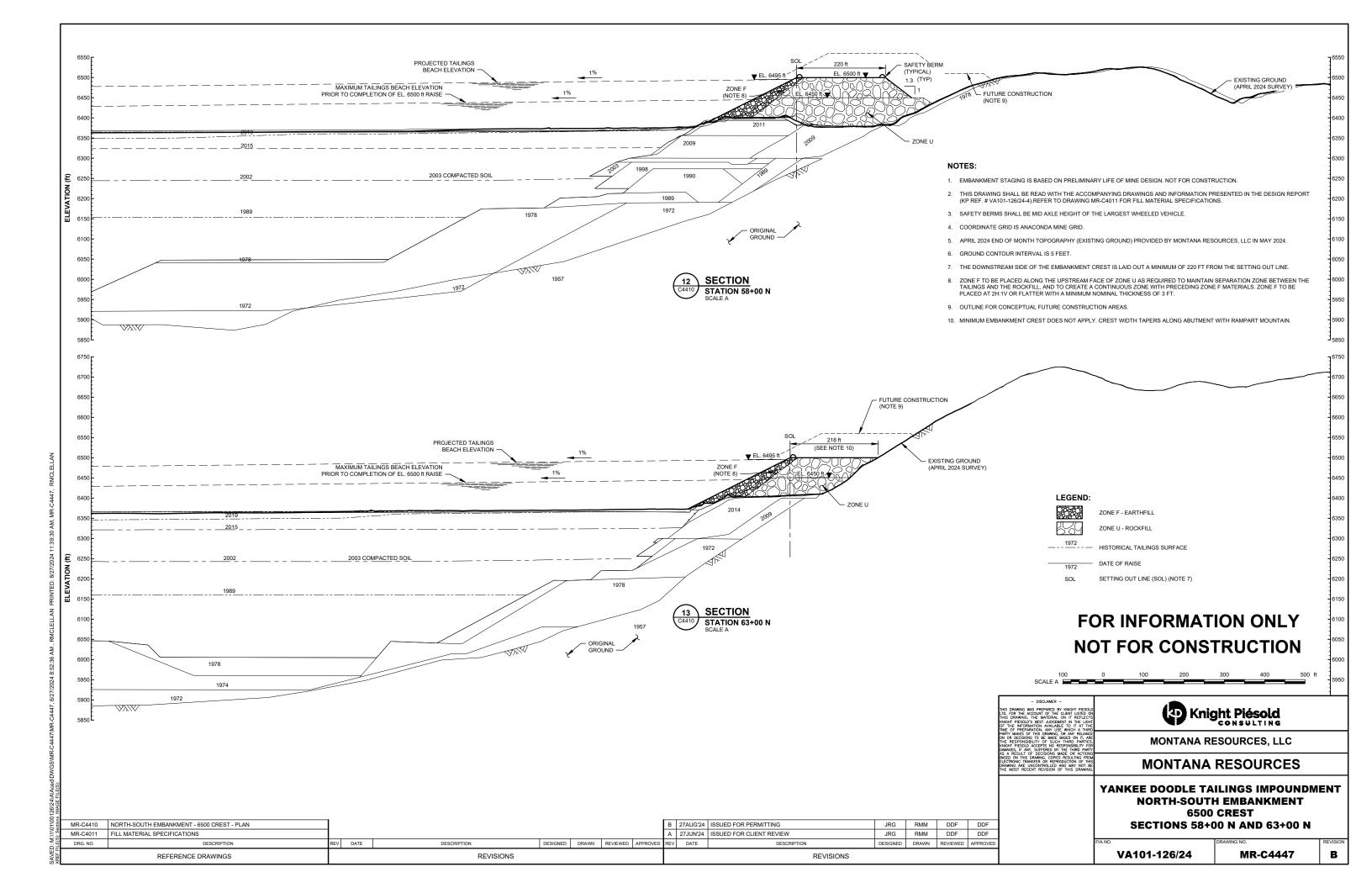
VA101-126/24 MR-C4442 В











APPENDIX C

Design Drawing Package - 6,560 ft Embankment Crest

Table C.1			
MR-C5020 Rev B	MR-C5310 Rev B	MR-C5335 Rev B	MR-C5441 Rev B
MR-C5110 Rev B	MR-C5311 Rev B	MR-C5336 Rev B	MR-C5442 Rev B
MR-C5111 Rev B	MR-C5330 Rev B	MR-C5337 Rev B	MR-C5443 Rev B
MR-C5129 Rev B	MR-C5331 Rev B	MR-C5338 Rev B	MR-C5444 Rev B
MR-C5130 Rev B	MR-C5332 Rev B	MR-C5410 Rev B	MR-C5445 Rev B
MR-C5131 Rev B	MR-C5333 Rev B	MR-C5411 Rev B	MR-C5446 Rev B
MR-C5132 Rev B	MR-C5334 Rev B	MR-C5440 Rev B	MR-C5447 Rev B





TABLE C.1

MONTANA RESOURCES, LLC MONTANA RESOURCES

YANKEE DOODLE TAILINGS IMPOUNDMENT LIFE OF MINE DESIGN REPORT 6560 CREST DRAWING LIST

Print Sep/13/24 13:58:38

Drawing Number	Rev.	Revision Date	Drawing Status	Print Sep/13/24 13:58:38 Drawing Title
MR-C5020	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - 6560 Crest - General Arrangement
MR-C5110	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - West Embankment - 6560 Crest - Plan
MR-C5111	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - West Embankment - 6560 Crest - Setting Out Details
MR-C5129	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - West Embankment - 6560 Crest - Sections 53+00 NW and 58+00 NW
MR-C5130	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - West Embankment - 6560 Crest - Sections 60+00 W and 69+50 W
MR-C5131	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - West Embankment - 6560 Crest - Sections 78+50 W and 82+80 W
MR-C5132	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - West Embankment - 6560 Crest - Sections 95+00 W and 108+40 W
MR-C5310	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6560 Crest - Plan
MR-C5311	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6560 Crest - Setting Out Details
MR-C5330	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6560 Crest - Section 0+00
MR-C5331	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6560 Crest - Section 03+00 W
MR-C5332	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6560 Crest - Section 08+00 W
MR-C5333	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6560 Crest - Section 12+00 W
MR-C5334	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6560 Crest - Section 18+00 NW
MR-C5335	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6560 Crest - Section 23+00 NW
MR-C5336	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6560 Crest - Section 28+00 NW
MR-C5337	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6560 Crest - Sections 33+00 NW and 38+00 NW
MR-C5338	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - East-West Embankment - 6560 Crest - Sections 43+00 NW and 48+00 NW
MR-C5410	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6560 Crest - Plan
MR-C5411	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6560 Crest - Setting Out Details
MR-C5440	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6560 Crest - Section 3+00 N
MR-C5441	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6560 Crest - Section 08+00 N
MR-C5442	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6560 Crest - Section 13+00 N
MR-C5443	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6560 Crest - Section 18+00 N and 23+00 N
MR-C5444	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6560 Crest - Section 28+00 N and 33+00 N
MR-C5445	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6560 Crest - Section 38+00 N and 43+00 N
MR-C5446	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6560 Crest - Section 48+00 N and 53+00 N
MR-C5447	В	27-Aug-24	Issued for Permitting	Yankee Doodle Tailings Impoundment - North-South Embankment - 6560 Crest - Section 58+00 N and 63+00 N

MR-C5447 B 27-Aug-24 Issued for Permitting Yankee Doodle Tailings Impoundment - North-South Embankment - 6560 Crest - Section 58+00 N and 63+00 N (knightpiesold.local\VA-Prj\$\1\01\00126\24\A\Report\4 - YDTI Life of Mine Design Report for A6560\Rev 0\Appendices\[App B and C Tables.xlsx]App C

0	13SEP'24	ISSUED WITH REPORT VA101-126/24-4		DDF
REV	DATE	DESCRIPTION	PREP'D	RVW'D

